

TRAFFIC IMPACT ANALYSIS

# Enclave – Maplewood

City of Maplewood, Minnesota

SEPTEMBER 24, 2022

PREPARED FOR:

Enclave Companies

St. Louis Park, Minnesota

PREPARED BY: **Westwood**

## Traffic Impact Analysis for

# Enclave – Maplewood

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9/24/2022

(Signature)

(date)

Minnesota Professional Engineering License 58308 Exp: 06/30/2024

Project Number: R0037066.00

Date: 9/24/2022

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## 1.0 Executive Summary

Westwood Professional Services, Inc. (Westwood) has been retained by Enclave Companies to prepare a Traffic Impact Analysis (TIA) for the Enclave – Maplewood project. The Enclave – Maplewood project includes 241 multifamily housing (mid-rise) dwelling units that are proposed for development on approximately +/- 7.97 acres located in Maplewood, Minnesota. Full buildout and occupancy of this project is expected to occur in the Year 2024.

This TIA will contain investigations of the potential impacts of the current development plan on the existing and planned roadway infrastructure and develop recommendations for mitigating the identified impacts.

This TIA contains the following analyses at the study intersections and driveways:

- Intersection Level of Service Analysis
- Left Turn and Right Turn Storage Lane Analysis
- Crash Analysis

These analyses will be performed within the following scenarios:

1. Year 2022 Existing Conditions
2. Year 2024 Background (No-Build) Conditions
3. Year 2024 Project Buildout Conditions

This TIA addresses anticipated future conditions at the following **two (2)** public street intersections:

1. County Road D and Southlawn Drive (Intersection #1)
2. Southlawn Drive and Beam Avenue (Intersection #2)

This study also addresses anticipated future conditions at **one (1)** private driveway:

1. Southlawn Drive and Project Driveway (Intersection #3)

The proposed Enclave – Maplewood project is anticipated to generate a total of 89 primary AM peak hour trips and 94 primary PM peak hour trips on an average weekday upon full buildout.

All intersections are anticipated to operate with acceptable LOS in the AM and PM peak hours.

All impacted left turn storage bays are anticipated to have adequate storage for Year 2024 Full Buildout Condition Scenario.

A total of 30 crashes were recorded at the two (2) intersections with crash data in the project area during the most recent five-year period. Of those 30 crashes, 7 crashes (23%) were property damage only crashes, 22 crashes (73%) were injury crashes with the remaining crash (4%) being fatal.

## 2.0 Introduction

Westwood Professional Services, Inc. (Westwood) has been retained by Enclave Companies to prepare a Traffic Impact Analysis (TIA) for the Enclave – Maplewood project. The Enclave – Maplewood project includes 241 multifamily housing (mid-rise) dwelling units that are proposed for development on approximately +/- 7.97 acres located in Maplewood, Minnesota. Full buildout and occupancy of this project is expected to occur in the Year 2024.

This TIA will contain investigations of the potential impacts of the current development plan on the existing and planned roadway infrastructure and develop recommendations for mitigating the identified impacts. The scope of the TIA is based on discussions with Ramsey County staff on July 7, 2022.

This TIA contains the following analyses at the study intersections and driveways:

- Intersection Level of Service Analysis
- Left Turn and Right Turn Storage Lane Analysis
- Crash Analysis

These analyses will be performed within the following scenarios:

4. Year 2022 Existing Conditions
5. Year 2024 Background (No-Build) Conditions
6. Year 2024 Project Buildout Conditions

Project information for the Enclave – Maplewood project is provided in **Appendix F**.

## 3.0 Existing Conditions

This section of the report details existing conditions in the immediate vicinity of the project site.

### 3.1 Existing Land Use

The Enclave – Maplewood project is generally bounded by County Road D to the north, Southlawn Drive to the west, a retail store to the east, and a senior living facility to the south. The location of the Enclave – Maplewood project is shown in **Figure 1**.

### 3.2 Regional Access

Primary regional access between the Minneapolis – St. Paul area and the proposed Enclave - Maplewood project is provided to and from Interstate 694 (I-694) via the White Bear Avenue interchange. Regional traffic can also travel between Maplewood Drive (US-61) and the proposed Enclave – Maplewood project via Country Road D and Beam Avenue. Site access is provided along arterial street connections such as County Road D and Southlawn Drive to these regional facilities.

### 3.3 Parcel Access

Access to the parcel is through an existing driveway on Southlawn Drive located approximately 260 feet south of County Road D. This existing driveway on Southlawn Drive will be realigned with the Ashley Furniture driveway located on the west side of Southlawn Drive.

The existing right-in/right-out driveway on Country Road D that is located approximately 300 feet east of Southlawn Drive is proposed to be closed as part of the development of this project.

### 3.4 Study Area Intersections

This TIA addresses anticipated future conditions at the following **two (2)** public street intersections:

3. County Road D and Southlawn Drive (Intersection #1)
4. Southlawn Drive and Beam Avenue (Intersection #2)

This study also addresses anticipated future conditions at **one (1)** private driveway:

2. Southlawn Drive and Project Driveway (Intersection #3)

The locations of each of these study intersections and private driveways are depicted in **Figure 1**.

### 3.5 Existing Lane Configuration and Traffic Control

Existing lane configurations and traffic control at the existing study intersections and future access intersections are depicted in **Figure 2**.

### 3.6 Existing Turning Movements

Existing AM and PM peak hour vehicular, pedestrian, and bicycle turning movement data were collected for the public street intersections identified in **Section 3.4**. The date and location of each count is summarized in **Table 1**. A summary of the peak hour count data at the study intersections is provided in **Figure 3**. The peak hour count data sheets are provided in **Appendix A**.

**Table 1 – Peak Hour Turning Movement Count Dates**

Intersection	Count Date
County Road D and Southlawn Drive (Intersection #1)	August 23, 2022
Southlawn Drive and Beam Avenue (Intersection #2)	May 17, 2022

Turning movement counts at Intersection 3 (Southlawn Drive and Ashley Furniture access/Future Project Access) were not collected. Trips were generated for the Ashley Furniture Store on the west side of Southlawn Drive and distributed through Intersection 3 as an estimate of existing traffic traveling into and out of the west leg of Intersection 3.



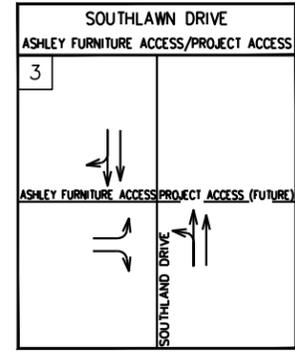
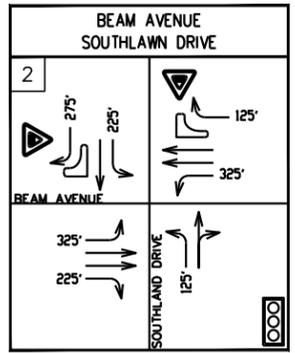
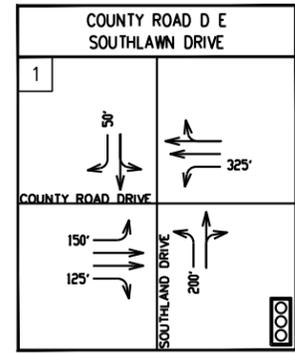
**LEGEND**

	STUDY INTERSECTION IDENTIFIER
	PROJECT DRIVEWAY IDENTIFIER

**ENCLAVE - MAPLEWOOD, MN**  
**STUDY AREA AND INTERSECTION IDENTIFIER**



Fri May 6 2022  
 Imagery © 2022 Nearmap, HERE  
 600 ft



**LEGEND**

	STUDY INTERSECTION IDENTIFIER		XXX' STORAGE LENGTH IF NO DIMENSION TURN LANE TRANSITIONS FROM A THRU LANE OR IS CONTINUOUS
	SIGNALIZED INTERSECTION		LANE CONFIGURATION
	STOP CONTROLLED APPROACH		PROJECT DRIVEWAY IDENTIFIER

**ENCLAVE - MAPLEWOOD, MN**  
**EXISTING YEAR 2022 LANE CONFIGURATION AND TRAFFIC CONTROL**



Fri May 6 2022  
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 600 ft

COUNTY ROAD D E SOUTHLAWN DRIVE	
1	
COUNTY ROAD DRIVE	

BEAM AVENUE SOUTHLAWN DRIVE	
2	
BEAM AVENUE	

SOUTHLAWN DRIVE ASHLEY FURNITURE ACCESS/PROJECT ACCESS	
3	
ASHLEY FURNITURE ACCESS	



LEGEND	
	PUBLIC STREET STUDY INTERSECTION IDENTIFIER
	PROJECT DRIVEWAY IDENTIFIER
	AM(PM) PEAK HOUR TRAFFIC VOLUME
	SIGNALIZED INTERSECTION

# ENCLAVE - MAPLEWOOD, MN YEAR 2022 EXISTING TRAFFIC VOLUMES

## 4.0 Future Conditions

This section of the report describes the anticipated buildout conditions for the Enclave – Maplewood project in the Year 2024.

### 4.1 Year 2024 Background Lane Configuration and Traffic Control

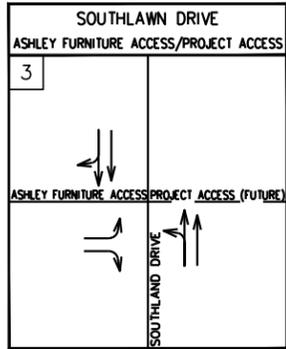
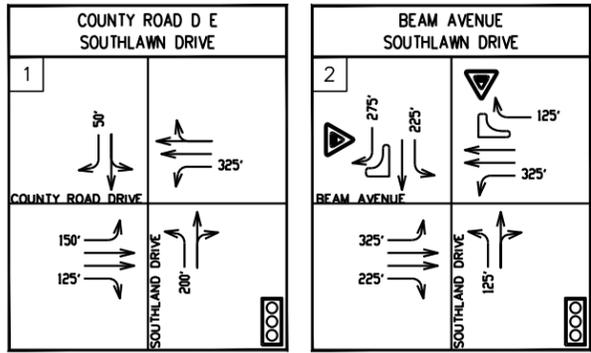
**Figure 4** illustrates the anticipated lane configuration and traffic control that are expected at the study area intersections in the background scenario during the buildout year of 2024. The background lane configuration and traffic control are expected to remain unchanged from the existing condition.

### 4.2 Year 2024 Background Traffic Volumes

Future baseline traffic volumes near the project site were generated for the anticipated project buildout year of 2024 to determine the impact of project traffic on the surrounding roadway network.

An average annual growth rate for the study area was calculated using historic Minnesota Department of Transportation (MnDOT) average annual daily traffic (AADT) data. An annual average growth rate of 0.8% was obtained from the evaluation of two (2) MnDOT count stations (#37586 and #37587). A conservative 2.0% growth rate was applied to the 2022 peak hour turning movement counts at each of the study intersections in **Figure 3** to generate traffic volumes for the background year of 2024.

The Year 2024 background traffic volumes are illustrated on **Figure 5**. Detailed calculations, along with information from the MnDOT Traffic Mapping Application, are included in **Appendix B**.



**LEGEND**

STUDY INTERSECTION IDENTIFIER	XXX' STORAGE LENGTH IF NO DIMENSION TURN LANE TRANSITIONS FROM A THRU LANE OR IS CONTINUOUS
SIGNALIZED INTERSECTION	LANE CONFIGURATION
STOP CONTROLLED APPROACH	PROJECT DRIVEWAY IDENTIFIER

**ENCLAVE - MAPLEWOOD, MN**  
**YEAR 2024 BACKGROUND (NO-BUILD) LANE CONFIGURATION AND TRAFFIC CONTROL**



COUNTY ROAD D E SOUTHLAWN DRIVE		BEAM AVENUE SOUTHLAWN DRIVE	
1			
COUNTY ROAD DRIVE	2(28) 0(7) 1(28)	1(28) 192(246) 54(54)	17(109) 14(17) 24(108)
SOUTHLAWN DRIVE	3(36) 218(434) 11(35)	5(23) 0(6) 2(80)	54(164) 249(517) 20(37)

SOUTHLAWN DRIVE ASHLEY FURNITURE ACCESS/PROJECT ACCESS	
3	
ASHLEY FURNITURE ACCESS	5(6) 63(95) 0(0)
SOUTHLAWN DRIVE	2(6) 0(0) 2(7)



LEGEND	
	PUBLIC STREET STUDY INTERSECTION IDENTIFIER
	PROJECT DRIVEWAY IDENTIFIER
	AM(PM) PEAK HOUR TRAFFIC VOLUME
	SIGNALIZED INTERSECTION
	STOP CONTROLLED INTERSECTION

**FIGURE 5**  
**Westwood**  
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 Minnetonka, MN 55343  
 westwoodps.com

**ENCLAVE - MAPLEWOOD, MN**  
**YEAR 2024 BACKGROUND (NO-BUILD) TRAFFIC VOLUMES**

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### 4.3 Project Trip Generation

The 11<sup>th</sup> Edition of the Institute of Transportation Engineers’ (ITE) Trip Generation Manual was used to estimate the number of vehicle trips that could be generated by the project. This manual is a standard reference used by municipalities and public agencies throughout the United States. The trip generation characteristics included in the manual are summarized by general land use type and are based on actual trip generation studies performed at numerous locations in areas of various populations.

Vehicular trip generation for the proposed Enclave – Maplewood project is based on average rates for Multifamily Housing (Mid-Rise) (ITE Land Use Code 221). The resulting trip generation is summarized in **Table 2**. Calculations are provided in **Appendix C**.

**Table 2 – Enclave – Maplewood Total Vehicular Trip Generation**

ITE Code	Land Use	Size	Daily Trips	AM Peak Hour			PM Peak Hour		
				In	Out	Total	In	Out	Total
221	<b>Multifamily Housing (Mid-Rise)</b>	241 du	1,094	20	69	89	57	37	94
<b>Total</b>			<b>1,094</b>	<b>20</b>	<b>69</b>	<b>89</b>	<b>57</b>	<b>37</b>	<b>94</b>

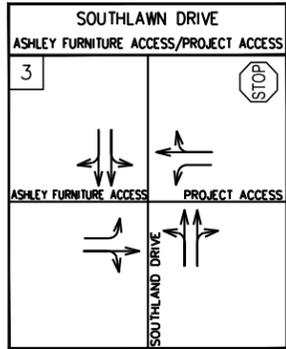
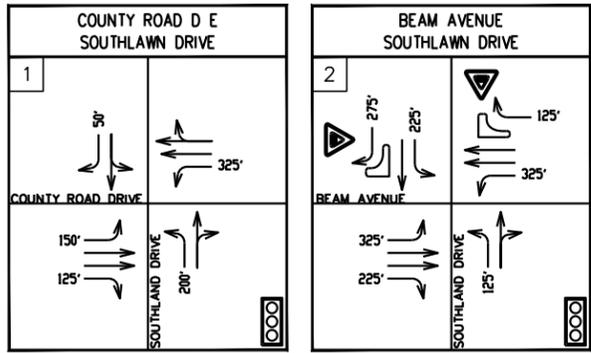
Sources: ITE Trip Generation Manual, 11<sup>th</sup> Edition

Pedestrian (i.e., walk + bike + transit) trip generation for the proposed Enclave – Maplewood project is based on average rates for Multifamily Housing (Mid-Rise) (ITE Land Use Code 221). The resulting trip generation is summarized in **Table 3**. Calculations are provided in **Appendix C**.

**Table 3 – Enclave – Maplewood Total Pedestrian Trip Generation**

ITE Code	Land Use	Size	Daily Trips	AM Peak Hour			PM Peak Hour		
				In	Out	Total	In	Out	Total
221	<b>Multifamily Housing (Mid-Rise)</b>	241 du	N/A	3	11	14	9	8	17
<b>Total</b>			<b>N/A</b>	<b>3</b>	<b>11</b>	<b>14</b>	<b>9</b>	<b>8</b>	<b>17</b>

Sources: ITE Trip Generation Manual, 11<sup>th</sup> Edition



**LEGEND**

STUDY INTERSECTION IDENTIFIER	XXX' STORAGE LENGTH IF NO DIMENSION TURN LANE TRANSITIONS FROM A THRU LANE OR IS CONTINUOUS
SIGNALIZED INTERSECTION	LANE CONFIGURATION
STOP CONTROLLED APPROACH	PROJECT DRIVEWAY IDENTIFIER

**ENCLAVE - MAPLEWOOD, MN  
YEAR 2024 PROJECT BUILDOUT LANE CONFIGURATION AND TRAFFIC CONTROL**

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600 ft

## City of Maplewood, Minnesota

#### 4.4 Project Trip Distribution

The directional distribution of project traffic quantifies the percentage of project-generated traffic that arrives and departs the project site in each direction by type of use. The project includes uses that generate primary project trips. The primary trip distribution quantifies the percentage of site-generated traffic that generally arrives and departs the project site along the same route. Primary trips are new trips to the street network and project driveways. **Figure 7** illustrates the anticipated primary project trip distribution for the Enclave – Maplewood project. The primary trip distribution was based on anticipated access restrictions and internal circulation through each project driveway, anticipated trip origins and destinations with the surrounding areas, and engineering judgment.

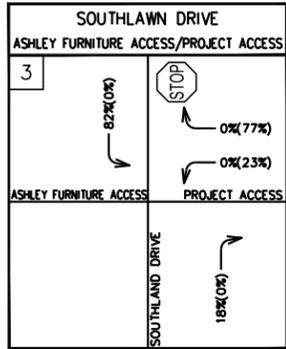
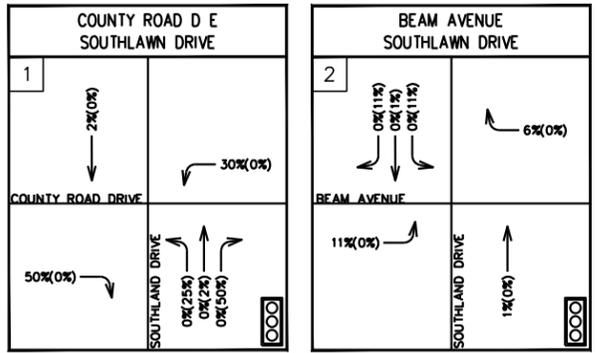
#### 4.5 Project Trip Assignment

Project traffic assignment was calculated by applying the primary trip distribution percentages found in **Figure 7** to the primary external vehicular project trip generation found in **Table 2**.

A Vistro model was prepared to distribute and assign the external project trips that were expected to be generated from the Enclave - Maplewood project throughout the study roadway network. The model uses zones to replicate specific areas or land uses within a development plan, links to replicate the roadway network, and gates at the boundary of the model to balance inbound and outbound traffic flows. The resulting traffic assignments at the study intersections and project driveways for the Enclave – Maplewood project are illustrated in **Figure 8** for the primary trip assignment. The inbound and outbound trip generation is rounded to the nearest whole number when assigned in Vistro to the distribution percentages. Therefore, the number of trips assigned to the study intersections may differ slightly from the total external trip generation in **Table 2**.

#### 4.6 Full Buildout Condition Traffic Volumes

The combined impact of background traffic and project traffic on the surrounding roadway network was determined by adding the project traffic assignment from **Figure 8** to the Year 2024 background traffic volumes in **Figure 5**. The traffic volumes illustrated in **Figure 9** represent the anticipated traffic conditions in the study area at the time the project is fully constructed and occupied in the Year 2024.



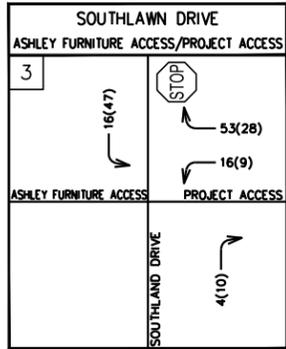
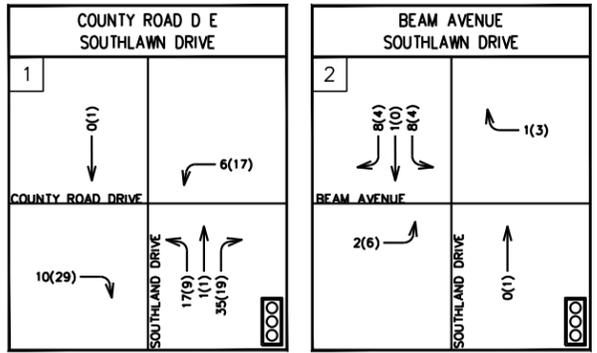
**LEGEND**

- X PUBLIC STREET STUDY INTERSECTION IDENTIFIER
- X PROJECT DRIVEWAY IDENTIFIER
- ← XX% (XX%) INBOUND (OUTBOND) DISTRIBUTION
- 00 SIGNALIZED INTERSECTION
- STOP STOP CONTROLLED INTERSECTION

**FIGURE 7**  
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 westwoodps.com

**ENCLAVE - MAPLEWOOD, MN  
 PROPOSED TRIP DISTRIBUTION**

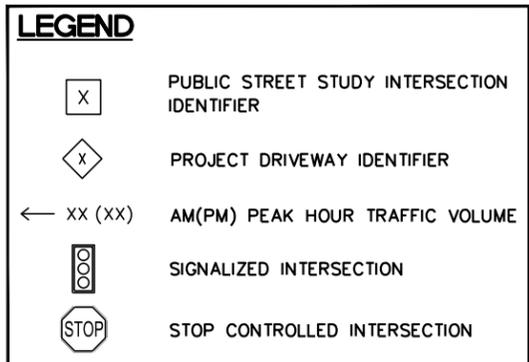
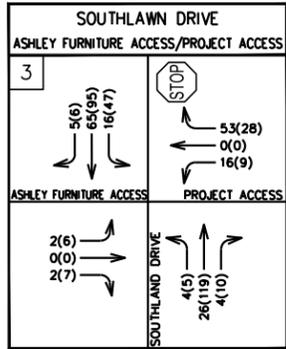
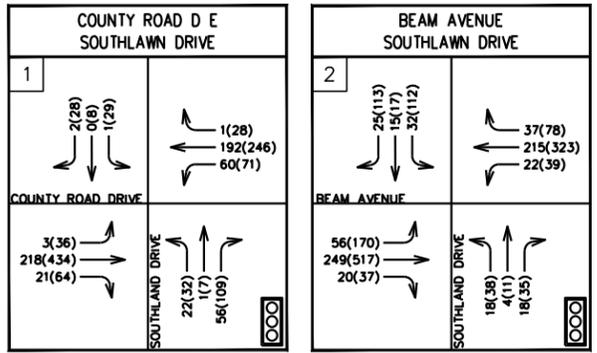
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 500 ft



**LEGEND**

- PUBLIC STREET STUDY INTERSECTION IDENTIFIER
- PROJECT DRIVEWAY IDENTIFIER
- AM(PM) PEAK HOUR TRAFFIC VOLUME
- SIGNALIZED INTERSECTION
- STOP CONTROLLED INTERSECTION

**ENCLAVE - MAPLEWOOD, MN  
PROPOSED TRIP ASSIGNMENT**



**FIGURE 9**  
**Westwood**  
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 westwoodps.com

**ENCLAVE - MAPLEWOOD, MN**  
**YEAR 2024 TOTAL TRAFFIC VOLUMES**

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## 5.0 Traffic Capacity Analysis

Analyses were conducted at the study area intersections identified in **Section 3.4** within the following scenarios:

1. Year 2022 Existing Conditions
2. Year 2024 Background (No-Build) Conditions
3. Year 2024 Project Buildout Conditions – Year 2024 Background (No-Build) Conditions with traffic generated by the 241 proposed lots

The purposes of these analyses are to identify potential capacity constraints in the existing and future study area street network and to quantify the impacts of the Enclave – Maplewood project.

### 5.1 Operational Analysis Methodology

The study intersections were analyzed using PTV Vistro Version 2022 (SP 0-3) Traffic Engineering Software. Level of Service (LOS) analyses within Vistro for signalized and stop controlled intersections were calculated and reported using the average total delay analysis methodology as presented in the Transportation Research Board’s Highway Capacity Manual (HCM 6<sup>th</sup> Edition). HCM 6<sup>th</sup> Edition describes LOS as “a qualitative stratification of a performance measure or measures that represent quality of service, measured on an A-F scale, with LOS A representing the best operating conditions from the traveler’s perspective and LOS F the worst.” **Table 4** summarizes the average total delay criteria for LOS A through F.

**Table 4 – Level of Service Criteria**

Level of Service	Signalized Intersection Average Total Delay (seconds/vehicle)	Stop Controlled / Roundabout Intersection Average Total Delay (seconds/vehicle)
A	≤10	10
B	>10 and ≤20	>10 and ≤15
C	>20 and ≤35	>15 and ≤25
D	>35 and ≤55	>25 and ≤35
E	>55 and ≤80	>35 and ≤50
F	>80	>50

*Highway Capacity Manual (HCM 6<sup>th</sup> Edition), Transportation Research Board*

## 5.2 Study Intersection Operational Analysis

Level of Service analyses for the study intersections are provided for the Year 2022 Existing Conditions, the Year 2024 Background Conditions, and the anticipated Year 2024 Full Buildout Conditions. The analyses are based on the lane configurations, traffic control and traffic volumes as illustrated in the following:

- Year 2022 Existing Conditions: **Figure 2** and **Figure 3**
- Year 2024 Background (No-Build) Conditions: **Figure 4** and **Figure 5**
- Year 2024 Project Buildout Conditions: **Figure 6** and **Figure 9**

Summary reports from the LOS analyses are provided for all study intersections in **Appendix D**. All intersections were analyzed using the lowest level of traffic control that provided an acceptable intersection LOS. Analysis for the signalized intersection of County Road D East and Southlawn Drive were completed with the signal timings provided by Ramsey County Public Works. The analysis for the signalized intersection of Beam Avenue and Southlawn Drive used an optimized cycle length with optimized splits, a 3-second yellow interval, and a 1-second all-red clearance interval. Results of the LOS analyses are summarized in **Table 5**.

All intersections are anticipated to operate with acceptable LOS in the AM and PM peak hours.

## 5.3 Left Turn Storage Bay Analysis

An analysis of all left turn storage bays that are expected to convey project-generated traffic at the study intersections was conducted for the following scenarios:

- Year 2022 Existing Conditions: **Figure 2** and **Figure 3**
- Year 2024 Background (No-Build) Conditions: **Figure 4** and **Figure 5**
- Year 2024 Project Buildout Conditions: **Figure 6** and **Figure 9**

The analysis uses the left turn queue lengths reported by Vistro (see **Appendix D**). **Table 5** summarizes the results of the analysis based on the anticipated traffic control required at full buildout of the Enclave – Maplewood project for defined left turn storage bays.

All impacted left turn storage bays are anticipated to have adequate storage for Year 2024 Full Buildout Condition Scenario.

INTERSECTION YEAR	TRAFFIC MANEUVER	TABLE 5. KEY INTERSECTION LOS ANALYSIS																																									
		AM					PM					AM					PM					AM					PM																
		LOS	Delay	Vistro Queue	V/C	Bay Length	LOS	Delay	Vistro Queue	V/C	Bay Length	LOS	Delay	Vistro Queue	V/C	Bay Length	LOS	Delay	Vistro Queue	V/C	Bay Length	LOS	Delay	Vistro Queue	V/C	Bay Length	LOS	Delay	Vistro Queue	V/C	Bay Length												
<b>INTERSECTION CONTROL</b>		<b>YEAR 2022 EXISTING - SIGNALIZED</b>														<b>YEAR 2024 (NO-BUILD) - SIGNALIZED</b>														<b>YEAR 2024 BUILDOUT - SIGNALIZED</b>													
INTERSECTION 1. COUNTY ROAD D EAST AND SOUTHLAWN DRIVE	Overall	A	6.40	-	0.040	200 ft	B	11.32	25 ft	0.220	200 ft	A	6.38	3 ft	0.040	200 ft	B	11.35	25 ft	0.220	200 ft	A	9.10	14 ft	0.160	200 ft	B	11.97	36 ft	0.300	200 ft												
	NBL	B	17.17	-	0.040	200 ft	C	28.70	25 ft	0.220	200 ft	B	17.20	3 ft	0.040	200 ft	C	28.67	25 ft	0.220	200 ft	B	19.84	14 ft	0.160	200 ft	C	29.18	36 ft	0.300	200 ft												
	NBT	N/A	--	-	-	-	N/A	--	-	-	-	B	16.49	-	-	-	C	21.26	-	-	-	B	15.03	-	-	-	C	21.14	-	-	-												
	NBR	B	16.46	12 ft	0.320	-	C	21.29	61 ft	0.360	-	B	16.49	12 ft	0.320	-	C	21.26	62 ft	0.360	-	B	15.03	31 ft	0.340	-	C	21.14	74 ft	0.410	-												
	SBL	B	17.16	1 ft	0.020	-	C	20.98	24 ft	0.180	-	B	17.19	1 ft	0.020	-	C	20.93	24 ft	0.180	-	B	17.17	1 ft	0.010	-	C	20.22	24 ft	0.180	-												
	SBT	N/A	--	-	-	-	N/A	--	-	-	-	B	17.19	-	-	-	C	20.93	-	-	-	B	17.17	-	-	-	C	20.22	-	-	-												
	SBR	B	14.87	1 ft	0.040	50ft	B	19.67	16 ft	0.100	50ft	B	14.90	1 ft	0.040	50ft	B	20 ft	16 ft	0.100	50ft	B	13.61	1 ft	0.010	50ft	B	19.10	15 ft	0.100	50ft												
	EBL	A	2.50	0 ft	0.000	150 ft	A	4.41	7 ft	0.050	150 ft	A	2.50	0 ft	0.000	150 ft	A	4.43	7 ft	0.050	150 ft	A	3.93	0 ft	-	150 ft	A	4.69	7 ft	0.050	150 ft												
	EBT	A	6.64	-	0.170	-	A	9.81	-	-	-	A	6.65	-	0.170	-	A	9.90	-	-	-	A	8.90	-	-	-	B	10.60	-	-	-												
	EBR	A	6.21	2 ft	0.030	125ft	A	8.55	16 ft	0.070	125ft	A	6.22	2 ft	0.030	125ft	A	8.61	16 ft	0.070	125ft	A	8.42	6 ft	0.050	125ft	A	9.65	31 ft	0.130	125ft												
	WBU						A	4.40	-	-	-	A	4.43	-	-	-	A	4.43	-	-	-	N/A	--	-	-	-	A	4.79	-	-	-												
	WBL	A	2.47	2 ft	0.050	325 ft	A	4.40	14 ft	0.070	325 ft	A	2.47	2 ft	0.050	325 ft	A	4.43	14 ft	0.070	325 ft	A	3.96	7 ft	7.330	325 ft	A	4.79	19 ft	0.090	325 ft												
	WBT	A	5.02	-	0.120	-	A	8.42	-	0.160	-	A	5.02	-	0.120	-	A	8.46	-	-	-	A	6.84	-	-	-	A	8.84	-	0.170	-												
	WBR	A	5.02	12 ft	0.120	-	A	8.44	44 ft	0.160	-	A	5.02	12 ft	0.120	-	A	8.48	44 ft	0.170	-	A	6.84	19 ft	18.790	-	A	8.86	45 ft	0.170	-												
<b>INTERSECTION CONTROL</b>		<b>YEAR 2022 EXISTING - SIGNALIZED</b>														<b>YEAR 2024 (NO-BUILD) - SIGNALIZED</b>														<b>YEAR 2024 BUILDOUT - SIGNALIZED</b>													
INTERSECTION 2. BEAM AVENUE AND SOUTHLAWN DRIVE	Overall	A	6.96	6 ft	0.070	125 ft	A	9.65	12 ft	0.110	125 ft	A	6.96	6 ft	0.070	125 ft	A	9.67	12 ft	0.110	125 ft	A	7.43	6 ft	0.070	125 ft	A	9.72	12 ft	0.110	125 ft												
	NBL	B	12.05	-	0.120	-	B	11.90	-	0.870	-	B	12.07	-	-	-	B	11.91	-	-	-	B	11.77	-	-	-	B	11.92	-	-	-												
	NBT	B	10.35	7 ft	0.120	-	B	10.18	14 ft	0.140	-	B	10.37	7 ft	0.120	-	B	10.19	10 ft	0.140	-	B	10.05	7 ft	0.110	-	B	10.21	15 ft	0.150	-												
	NBR	B	10.35	7 ft	0.120	-	B	10.18	14 ft	0.140	-	B	10.37	7 ft	0.120	-	B	10.19	10 ft	0.140	-	B	10.05	7 ft	0.110	-	B	10.21	15 ft	0.150	-												
	SBU	N/A	--	-	-	-	B	13.27	31 ft	-	-		--	-	0.090	-	B	13.29	13 ft	-	-	N/A	--	-	-	-	B	13.41	-	-	-												
	SBL	B	12.29	7 ft	0.090	225 ft	B	13.27	31 ft	0.260	225 ft	B	12.31	7 ft	0.090	225 ft	B	13.29	13 ft	0.260	225 ft	B	12.11	9 ft	0.110	225 ft	B	13.41	33 ft	0.270	225 ft												
	SBT	B	10.17	-	0.080	-	A	9.85	-	-	-	B	10.19	-	-	-	A	9.85	-	-	-	A	9.92	-	-	-	A	9.87	-	-	-												
	SBR	B	10.24	5 ft	0.090	275 ft	B	11.24	37 ft	0.370	275 ft	B	10.25	5 ft	0.090	275 ft	B	11.27	11 ft	0.370	275 ft	B	10.07	7 ft	0.110	275 ft	B	11.35	39 ft	-	275 ft												
	EBU	A	3.11	-	0.080	-	A	5.10	-	-	-	A	3.11	-	0.080	-	A	5.12	5 ft	-	-	A	3.50	-	-	-	A	5.16	0.380	-	-												
	EBL	A	3.11	3 ft	0.080	325 ft	A	5.10	21 ft	0.230	325 ft	A	3.11	3 ft	0.080	325 ft	A	5.12	5 ft	0.230	325 ft	A	3.50	5 ft	0.090	325 ft	A	5.16	22 ft	-	325 ft												
	EBT	A	6.46	-	0.230	-	A	9.52	-	-	-	A	6.46	-	-	-	A	9.55	-	-	-	A	6.97	-	0.240	-	A	9.56	-	0.240	-												
	EBR	A	5.99	3 ft	0.060	225 ft	A	8.17	9 ft	0.100	225 ft	A	5.98	3 ft	0.060	225 ft	A	8.18	8 ft	0.100	225 ft	A	6.45	4 ft	0.060	225 ft	A	8.19	9 ft	-	225 ft												
	WBU	A	3.01	-	0.040	-	A	4.75	-	-	-	A	3.01	-	0.040	-	A	4.76	5 ft	-	-	A	3.38	-	-	-	A	4.78	-	-	-												
	WBL	A	3.01	1 ft	0.040	325 ft	A	4.75	5 ft	0.070	325 ft	A	3.01	1 ft	0.040	325 ft	A	4.76	5 ft	0.070	325 ft	A	3.38	0 ft	0.040	325 ft	A	4.78	5 ft	0.070	325 ft												
WBT	A	7.22	-	0.240	-	B	10.67	-	-	-	A	7.24	-	-	-	B	10.70	-	-	-	A	7.82	-	0.260	-	B	10.75	-	-	-													
WBR	A	6.85	7 ft	0.100	125 ft	B	10.39	10 ft	0.250	125 ft	A	6.86	7 ft	0.100	125 ft	B	10.41	10 ft	0.260	125 ft	A	7.41	0 ft	0.110	125 ft	B	10.52	27 ft	0.272	125 ft													
<b>INTERSECTION CONTROL</b>		<b>YEAR 2022 EXISTING</b>														<b>YEAR 2024 (NO-BUILD)</b>														<b>YEAR 2024 BUILDOUT - TWO-WAY STOP CONTROL</b>													
INTERSECTION 3. SOUTHLAWN DRIVE AND ASHLEY FURNITURE ACCESS/PROJECT ACCESS	Overall	Not Applicable														Not Applicable														A	4.09	-	-	-	A	2.63	-	-	-				
	NBL	Not Applicable														Not Applicable														A	7.37	0 ft	0.000	-	A	7.44	0 ft	0.000	-				
	NBT	Not Applicable														Not Applicable														N/A	--	-	-	-	N/A	--	0 ft	-	-				
	NBR	Not Applicable														Not Applicable														N/A	--	-	-	-	N/A	--	0 ft	-	-				
	SBL	Not Applicable														Not Applicable														A	7.30	1 ft	0.010	-	A	7.56	2 ft	0.040	-				
	SBT	Not Applicable														Not Applicable														N/A	--	-	-	-	N/A	--	-	-	-				
	SBR	Not Applicable														Not Applicable														N/A	--	-	-	-	N/A	--	0 ft	-	-				
	EBL	Not Applicable														Not Applicable														A	9.66	0 ft	0.000	-	B	11.06	1 ft	0.010	-				
	EBT	Not Applicable														Not Applicable														N/A	--	-	0.000	-	N/A	--	-	0.000	-				
	EBR	Not Applicable														Not Applicable														A	8.52	0 ft	0.000	-	A	8.63	0 ft	0.010	-				
	WBL	Not Applicable														Not Applicable														A	9.34	2 ft	0.020	-	B	11.08	1 ft	0.010	-				
	WBT	Not Applicable														Not Applicable														N/A	--	-	0.000	-	N/A	--	-	0.000	-				
	WBR	Not Applicable														Not Applicable														A	8.59	4 ft	0.050	-	A	8.80	3 ft	0.030	-				

KEY:  
LOS = Level of Service  
Delay = Average Seconds of Delay Per Vehicle  
Vistro Queue = Vistro Software Estimated Turn Left Queue  
V/C = Volume to Capacity Ratio  
LT Bay Length = Number of Defined Left Turn Lanes and Storage Length  
AM = AM Peak Hour of Adjacent Street  
PM = PM Peak Hour of Adjacent Street

## 6.0 Crash Analysis

Crash data was requested for the two (2) study intersections identified in Section 3.4 from the Ramsey County Public Works for the most recent five-year reporting period. The crash data received from Ramsey County Public Works is included in **Appendix E** and is summarized in **Table 6**.

**Table 6 – Crash Data Summary**

<b>INT #</b>	<b>Intersection</b>	<b>Total Collisions</b>	<b>Property Damage Only</b>	<b>Injury</b>	<b>Fatality</b>
1	County Road D and Southlawn Drive	4	1	2	1
2	Southlawn Drive and Beam Avenue	26	6	20	0
<b>TOTAL</b>		30	7	22	1

A total of 30 crashes were recorded at the two (2) intersections with crash data in the project area during the most recent five-year period. Of those 30 crashes, 7 crashes (23%) were property damage only crashes, 22 crashes (73%) were injury crashes with the remaining crash (4%) being fatal.

## 7.0 Conclusions and Recommendations

All intersections are anticipated to operate with acceptable LOS in the AM and PM peak hours.

All impacted left turn storage bays are anticipated to have adequate storage for Year 2024 Full Buildout Condition Scenario.

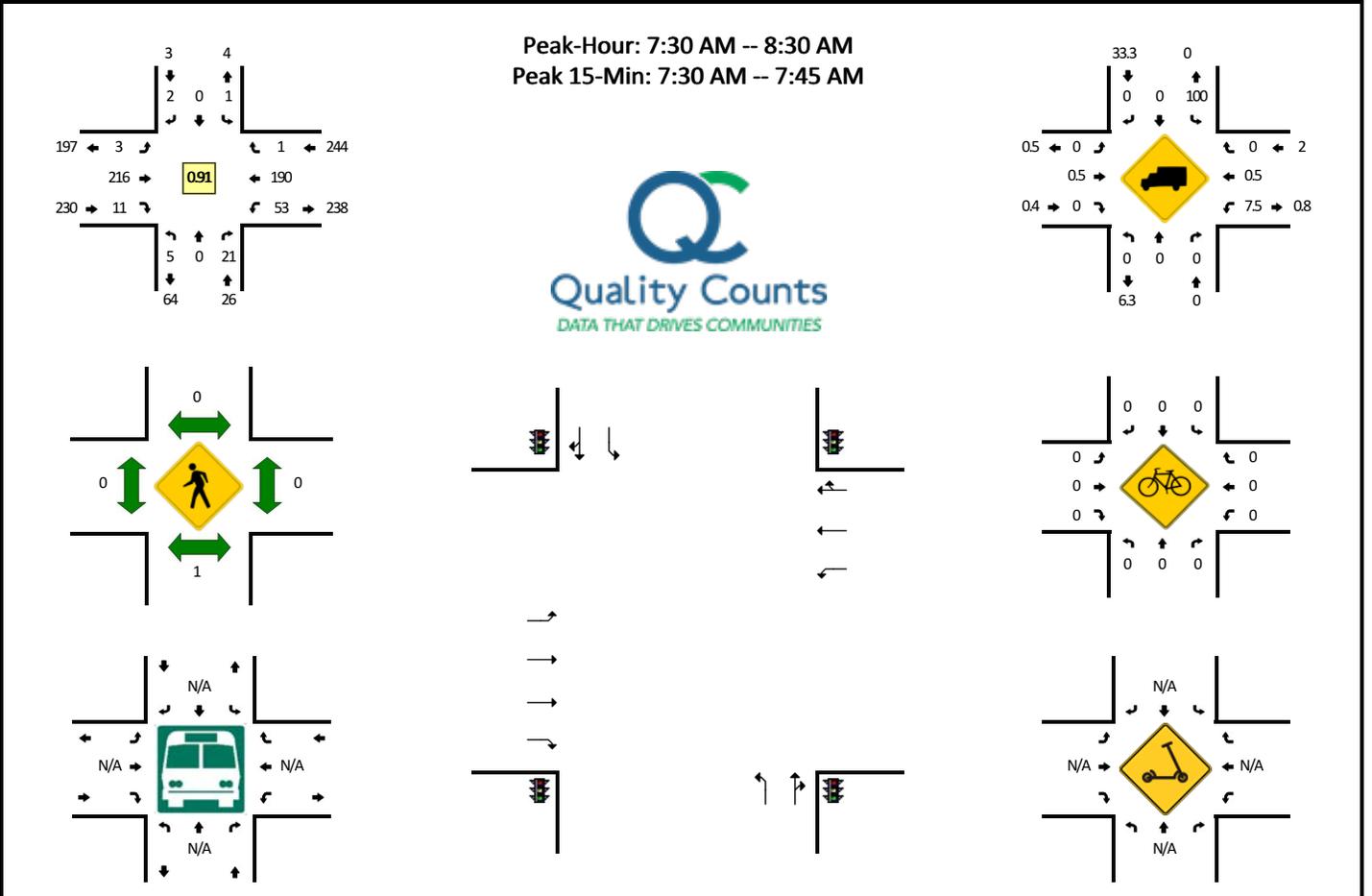
A total of 30 crashes were recorded at the two (2) intersections with crash data in the project area during the most recent five-year period. Of those 30 crashes, 7 crashes (23%) were property damage only crashes, 22 crashes (73%) were injury crashes with the remaining crash (4%) being fatal.

# APPENDIX A

## Existing Peak Hour Traffic Count Data

**LOCATION:** Southlawn Dr -- County Rd D  
**CITY/STATE:** Maplewood, MN

**QC JOB #:** 15874401  
**DATE:** Tue, Aug 23 2022

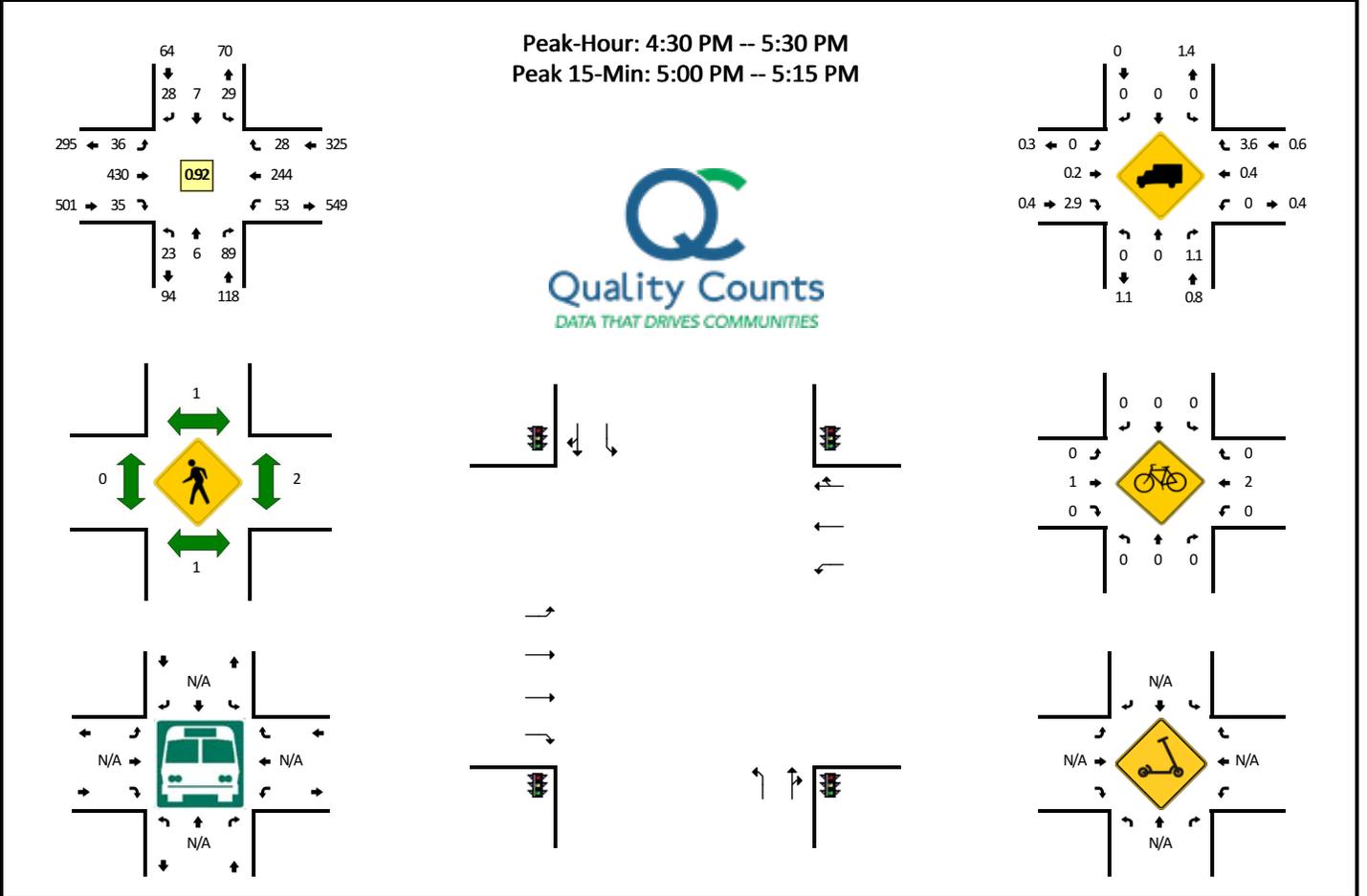


15-Min Count Period Beginning At	Southlawn Dr (Northbound)				Southlawn Dr (Southbound)				County Rd D (Eastbound)				County Rd D (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
6:00 AM	2	0	0	0	0	0	0	0	0	6	0	0	4	12	0	0	24	
6:15 AM	1	0	0	0	0	0	0	0	0	15	1	0	7	18	0	0	42	
6:30 AM	0	0	3	0	0	0	0	0	0	13	1	0	5	50	0	0	72	
6:45 AM	2	0	5	0	0	0	0	0	0	11	2	0	22	79	0	0	121	259
7:00 AM	1	0	3	0	0	0	0	0	0	11	3	1	8	27	1	0	55	290
7:15 AM	1	0	7	0	0	0	1	0	1	33	1	0	18	43	0	0	105	353
7:30 AM	2	0	1	0	0	0	0	0	1	60	3	0	17	54	0	0	138	419
7:45 AM	0	0	5	0	1	0	1	0	1	51	4	0	15	47	1	0	126	424
8:00 AM	1	0	6	0	0	0	0	0	0	56	3	0	9	41	0	0	116	485
8:15 AM	2	0	9	0	0	0	1	0	1	49	1	0	12	48	0	0	123	503
8:30 AM	3	0	10	0	0	0	0	0	1	25	6	0	9	52	2	0	108	473
8:45 AM	2	0	8	0	0	1	2	0	0	38	7	0	15	37	0	0	110	457
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
All Vehicles	8	0	4	0	0	0	0	0	4	240	12	0	68	216	0	0	552	
Heavy Trucks	0	0	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	
Buses																		
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Scoters																		

*Comments:*

**LOCATION:** Southlawn Dr -- County Rd D  
**CITY/STATE:** Maplewood, MN

**QC JOB #:** 15874402  
**DATE:** Tue, Aug 23 2022



15-Min Count Period Beginning At	Southlawn Dr (Northbound)				Southlawn Dr (Southbound)				County Rd D (Eastbound)				County Rd D (Westbound)				Total	Hourly Totals	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
3:00 PM	8	3	32	0	7	1	5	0	10	93	13	0	24	63	10	2	271		
3:15 PM	10	2	35	0	8	3	4	0	9	68	13	2	10	70	1	0	235		
3:30 PM	6	3	32	0	11	7	5	0	2	93	12	0	16	70	8	0	265		
3:45 PM	5	5	21	0	4	3	6	0	5	96	9	0	8	60	9	0	231	1002	
4:00 PM	6	2	29	0	11	3	5	0	9	110	10	0	10	59	10	0	264	995	
4:15 PM	5	1	35	0	6	3	5	0	7	75	10	0	14	55	9	0	225	985	
4:30 PM	10	1	24	0	7	2	8	0	9	122	6	0	14	61	5	0	269	989	
4:45 PM	5	2	26	0	6	1	7	0	9	78	6	0	6	53	9	1	209	967	
5:00 PM	6	0	21	0	8	0	5	0	11	122	10	0	18	66	7	0	274	977	
5:15 PM	2	3	18	0	8	4	8	0	7	108	13	0	14	64	7	0	256	1008	
5:30 PM	10	2	27	0	8	3	5	0	9	79	13	0	9	71	4	0	240	979	
5:45 PM	9	2	19	0	10	3	2	0	4	79	9	0	15	57	4	0	213	983	
Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total		
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
All Vehicles	24	0	84	0	32	0	20	0	44	488	40	0	72	264	28	0	1096		
Heavy Trucks	0	0	4		0	0	0		0	0	0		0	0	0		4		
Buses																			
Pedestrians	0	0			0	0			0	0			0	4			4		
Bicycles	0	0			0	0			0	0			0	4	0		4		
Scoters																			

*Comments:*

# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Site Code:**  
**Study Date:** 05/17/2022

**Cars and Peds**

Time	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound						Int Total
	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	
05:00		0	0	0	0	0		1	2	0	1	4		0	0	1	0	1		0	4	2	0	6	11
05:15		1	0	0	0	1		1	6	0	0	7		0	0	0	0	0		0	3	3	0	6	14
05:30		0	0	1	0	1		1	11	0	0	12		0	0	2	0	2		0	3	3	1	7	22
05:45		1	0	3	0	4		1	16	1	0	18		0	2	3	0	5		1	10	8	0	19	46
<b>Total</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>4</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>4</b>	<b>35</b>	<b>1</b>	<b>1</b>	<b>41</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>6</b>	<b>0</b>	<b>8</b>	<b>0</b>	<b>1</b>	<b>20</b>	<b>16</b>	<b>1</b>	<b>38</b>	<b>93</b>
06:00		1	2	1	0	4		3	21	5	1	30		1	1	2	0	4		1	7	8	1	17	55
06:15		4	2	1	0	7		5	26	0	0	31		0	1	0	0	1		1	10	9	2	22	61
06:30		4	0	1	0	5		6	36	0	0	42		2	0	2	0	4		5	18	8	0	31	82
06:45		2	2	2	0	6		6	60	3	3	72		3	2	4	0	9		3	46	10	0	59	146
<b>Total</b>	<b>0</b>	<b>11</b>	<b>6</b>	<b>5</b>	<b>0</b>	<b>22</b>	<b>0</b>	<b>20</b>	<b>143</b>	<b>8</b>	<b>4</b>	<b>175</b>	<b>0</b>	<b>6</b>	<b>4</b>	<b>8</b>	<b>0</b>	<b>18</b>	<b>0</b>	<b>10</b>	<b>81</b>	<b>35</b>	<b>3</b>	<b>129</b>	<b>344</b>
07:00		0	2	5	0	7		8	51	1	0	60		1	0	4	0	5		1	22	7	2	32	104
07:15		4	0	3	0	7		7	59	0	3	69		0	1	2	0	3		0	26	13	4	43	122
07:30		2	0	2	0	4		4	57	2	3	66		2	0	6	0	8		0	55	15	3	73	151
07:45		4	1	6	0	11		7	65	2	2	76		2	0	3	0	5		6	51	14	1	72	164
<b>Total</b>	<b>0</b>	<b>10</b>	<b>3</b>	<b>16</b>	<b>0</b>	<b>29</b>	<b>0</b>	<b>26</b>	<b>232</b>	<b>5</b>	<b>8</b>	<b>271</b>	<b>0</b>	<b>5</b>	<b>1</b>	<b>15</b>	<b>0</b>	<b>21</b>	<b>0</b>	<b>7</b>	<b>154</b>	<b>49</b>	<b>10</b>	<b>220</b>	<b>541</b>
08:00		3	0	2	0	5		6	71	2	2	81		5	0	4	0	9		3	39	9	1	52	147
08:15		4	4	6	0	14		8	57	3	1	69		5	0	3	0	8		3	76	11	2	92	183
08:30		4	6	8	0	18		9	43	3	3	58		4	4	4	0	12		6	65	14	0	85	173
08:45		6	4	8	0	18		13	42	7	1	63		4	0	7	0	11		8	67	19	1	95	187
<b>Total</b>	<b>0</b>	<b>17</b>	<b>14</b>	<b>24</b>	<b>0</b>	<b>55</b>	<b>0</b>	<b>36</b>	<b>213</b>	<b>15</b>	<b>7</b>	<b>271</b>	<b>0</b>	<b>18</b>	<b>4</b>	<b>18</b>	<b>0</b>	<b>40</b>	<b>0</b>	<b>20</b>	<b>247</b>	<b>53</b>	<b>4</b>	<b>324</b>	<b>690</b>
09:00		3	3	6	0	12		16	49	3	1	69		6	1	3	0	10		6	67	15	0	88	179
09:15		9	2	10	0	21		11	57	1	2	71		1	3	5	0	9		2	66	19	3	90	191
09:30		8	2	13	0	23		14	70	4	1	89		7	5	7	0	19		3	63	23	2	91	222
09:45		15	1	12	0	28		20	78	6	1	105		5	1	5	0	11		2	90	32	1	125	269
<b>Total</b>	<b>0</b>	<b>35</b>	<b>8</b>	<b>41</b>	<b>0</b>	<b>84</b>	<b>0</b>	<b>61</b>	<b>254</b>	<b>14</b>	<b>5</b>	<b>334</b>	<b>0</b>	<b>19</b>	<b>10</b>	<b>20</b>	<b>0</b>	<b>49</b>	<b>0</b>	<b>13</b>	<b>286</b>	<b>89</b>	<b>6</b>	<b>394</b>	<b>861</b>

Time	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound						Int Total
	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	
10:00		16	3	19	0	38		19	94	6	4	123		3	0	4	0	7		6	87	32	4	129	297
10:15		17	1	17	0	35		15	71	6	0	92		3	2	4	0	9		2	85	28	2	117	253
10:30		20	2	10	0	32		16	73	5	1	95		9	0	6	0	15		4	84	43	2	133	275
10:45		14	2	10	0	26		24	94	3	3	124		4	0	4	0	8		8	126	27	1	162	320
Total	0	67	8	56	0	131	0	74	332	20	8	434	0	19	2	18	0	39	0	20	382	130	9	541	1145
11:00		25	4	24	0	53		19	77	9	3	108		4	3	7	0	14		11	92	33	0	136	311
11:15		22	4	26	0	52		19	82	10	0	111		5	7	6	0	18		4	127	41	2	174	355
11:30		26	5	24	0	55		19	74	6	4	103		13	5	8	0	26		6	106	34	1	147	331
11:45		32	5	23	0	60		21	91	5	0	117		4	4	3	0	11		2	134	40	1	177	365
Total	0	105	18	97	0	220	0	78	324	30	7	439	0	26	19	24	0	69	0	23	459	148	4	634	1362
12:00		23	4	31	0	58		20	88	10	2	120		8	4	7	0	19		5	121	39	0	165	362
12:15		23	2	20	0	45		24	113	10	2	149		8	3	10	0	21		8	108	26	1	143	358
12:30		23	3	24	0	50		21	98	15	0	134		2	5	7	0	14		7	104	39	4	154	352
12:45		18	3	24	0	45		18	101	13	3	135		6	4	9	0	19		5	95	35	5	140	339
Total	0	87	12	99	0	198	0	83	400	48	7	538	0	24	16	33	0	73	0	25	428	139	10	602	1411
13:00		26	4	33	0	63		20	87	8	1	116		7	2	5	0	14		4	104	39	0	147	340
13:15		19	3	24	0	46		16	120	8	3	147		5	5	2	0	12		3	97	28	2	130	335
13:30		24	5	24	0	53		19	108	12	2	141		7	2	5	0	14		4	112	34	1	151	359
13:45		32	7	33	0	72		17	85	10	5	117		6	5	9	0	20		4	105	33	2	144	353
Total	0	101	19	114	0	234	0	72	400	38	11	521	0	25	14	21	0	60	0	15	418	134	5	572	1387
14:00		28	3	25	0	56		20	92	6	1	119		11	3	5	0	19		5	107	30	2	144	338
14:15		14	5	22	0	41		27	132	8	1	168		5	2	13	0	20		4	107	29	1	141	370
14:30		28	5	25	0	58		8	93	4	2	107		5	4	11	0	20		8	106	25	3	142	327
14:45		22	2	27	0	51		18	86	6	4	114		7	3	6	0	16		4	97	41	3	145	326
Total	0	92	15	99	0	206	0	73	403	24	8	508	0	28	12	35	0	75	0	21	417	125	9	572	1361
15:00		19	8	31	0	58		25	90	6	0	121		4	5	12	0	21		7	98	31	1	137	337
15:15		26	3	30	0	59		23	89	6	3	121		4	3	7	0	14		6	117	32	3	158	352
15:30		17	2	21	0	40		20	90	6	4	120		12	2	14	0	28		3	135	34	2	174	362
15:45		19	5	28	0	52		21	74	7	2	104		11	4	9	0	24		9	125	30	2	166	346
Total	0	81	18	110	0	209	0	89	343	25	9	466	0	31	14	42	0	87	0	25	475	127	8	635	1397
16:00		24	7	24	0	55		17	78	9	1	105		9	1	9	0	19		13	128	46	1	188	367
16:15		24	2	27	1	54		28	87	6	1	122		10	6	12	0	28		6	130	33	1	170	374
16:30		39	5	28	0	72		12	91	9	1	113		10	1	9	0	20		12	137	39	2	190	395
16:45		21	3	20	0	44		19	64	12	0	95		6	2	8	0	16		6	117	40	2	165	320

Time	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound						Int Total
	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	
<b>Total</b>	0	108	17	99	1	225	0	76	320	36	3	435	0	35	10	38	0	83	0	37	512	158	6	713	1456
17:00		21	4	14	1	40		23	88	7	3	121		6	1	7	0	14		13	133	40	0	186	361
17:15		19	1	19	0	39		16	98	9	1	124		5	3	12	0	20		7	124	45	2	178	361
17:30		22	4	18	0	44		26	78	5	0	109		4	4	10	0	18		16	134	37	1	188	359
17:45		20	8	21	0	49		22	75	14	1	112		7	1	9	0	17		8	105	47	3	163	341
<b>Total</b>	0	82	17	72	1	172	0	87	339	35	5	466	0	22	9	38	0	69	0	44	496	169	6	715	1422
18:00		30	4	25	0	59		30	91	4	2	127		5	0	8	0	13		5	96	44	2	147	346
18:15		27	2	23	0	52		30	86	11	0	127		3	7	14	0	24		7	89	40	1	137	340
18:30		29	3	25	0	57		23	85	15	1	124		7	3	17	0	27		2	105	56	2	165	373
18:45		26	2	21	0	49		14	71	9	2	96		10	7	6	0	23		7	89	48	0	144	312
<b>Total</b>	0	112	11	94	0	217	0	97	333	39	5	474	0	25	17	45	0	87	0	21	379	188	5	593	1371
19:00		25	3	29	0	57		16	59	7	2	84		5	4	7	0	16		2	66	37	1	106	263
19:15		34	3	25	0	62		13	70	6	2	91		4	7	6	0	17		7	48	17	5	77	247
19:30		22	1	15	0	38		18	78	4	3	103		3	0	6	0	9		5	67	27	1	100	250
19:45		23	1	21	0	45		14	80	6	4	104		7	0	15	0	22		5	56	22	0	83	254
<b>Total</b>	0	104	8	90	0	202	0	61	287	23	11	382	0	19	11	34	0	64	0	19	237	103	7	366	1014
20:00		26	2	21	0	49		17	61	9	6	93		5	2	8	0	15		6	54	16	2	78	235
20:15		22	3	19	1	45		7	41	6	3	57		7	0	6	0	13		8	55	6	2	71	186
20:30		12	0	16	0	28		7	37	2	1	47		2	2	9	0	13		4	38	10	0	52	140
20:45		8	0	7	0	15		4	24	4	2	34		3	2	6	0	11		2	27	12	0	41	101
<b>Total</b>	0	68	5	63	1	137	0	35	163	21	12	231	0	17	6	29	0	52	0	20	174	44	4	242	662
21:00		10	1	8	0	19		6	39	3	0	48		1	0	2	0	3		1	34	3	0	38	108
21:15		13	0	8	0	21		3	28	0	2	33		1	0	1	0	2		1	22	4	0	27	83
21:30		9	2	3	0	14		0	18	5	0	23		12	3	5	0	20		2	17	5	0	24	81
21:45		11	0	8	0	19		1	13	0	0	14		3	0	4	0	7		2	20	5	2	29	69
<b>Total</b>	0	43	3	27	0	73	0	10	98	8	2	118	0	17	3	12	0	32	0	6	93	17	2	118	341
<b>Grand Total</b>	0	1125	182	1110	3	2420	0	982	4619	390	113	6104	0	336	154	436	0	926	0	327	5258	1724	99	7408	16858
<b>Appr %</b>		46.5	7.5	45.9	0.1			16.1	75.7	6.4	1.9			36.3	16.6	47.1	0			4.4	71	23.3	1.3		
<b>Total %</b>		6.7	1.1	6.6	0			5.8	27.4	2.3	0.7			2	0.9	2.6	0			1.9	31.2	10.2	0.6		

	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound							
Time	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Peds	Right	Thru	Left	U-Turn	Appr Total	Int Total	
AM Pk Hr		11:45	11:45	11:45	11:45	11:45		11:45	11:45	11:45	11:45	11:45		11:45	11:45	11:45	11:45	11:45		11:45	11:45	11:45	11:45	11:45	11:45	11:45
AM Pk Vol		101	14	98	0	213		86	390	40	4	520		22	16	27	0	65		22	467	144	6	639	1437	
AM PHF		0.789	0.700	0.790	NaN	0.888		0.896	0.863	0.667	0.500	0.872		0.688	0.800	0.675	NaN	0.774		0.688	0.871	0.900	0.375	0.903	0.984	
PM Pk Hr		15:45	15:45	15:45	15:45	15:45		15:45	15:45	15:45	15:45	15:45		15:45	15:45	15:45	15:45	15:45		15:45	15:45	15:45	15:45	15:45	15:45	15:45
PM Pk Vol		106	19	107	1	233		78	330	31	5	444		40	12	39	0	91		40	520	148	6	714	1482	
PM PHF		0.679	0.679	0.955	0.250	0.809		0.696	0.907	0.861	0.625	0.910		0.909	0.500	0.813	NaN	0.813		0.769	0.949	0.804	0.750	0.939	0.938	

# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Trucks and Bikes**

**Site Code:**  
**Study Date:** 05/17/2022

Time	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound						Int Total						
	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total							
05:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
05:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
05:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
05:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
06:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
06:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
06:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
06:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
07:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
07:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
07:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
07:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
08:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
08:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
08:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
08:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
09:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
09:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
09:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
09:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>

Time	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound						Int Total	
	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total		
10:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
10:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
10:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
10:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
11:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
11:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
11:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
12:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
12:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
12:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
13:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
13:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
13:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
13:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
14:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
14:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
14:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
14:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
15:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
15:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
15:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
15:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
16:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
16:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
16:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
16:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0

Time	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound						Int Total	
	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total		
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
17:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
17:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
17:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
17:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
18:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
18:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
18:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
18:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
19:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
19:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
19:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
20:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
20:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
20:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
20:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
21:00		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
21:15		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
21:30		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
21:45		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Appr %		-2	-2	-2	-2			-2	-2	-2	-2			-2	-2	-2	-2			-2	-2	-2	-2			
Total %		-3	-3	-3	-3			-3	-3	-3	-3			-3	-3	-3	-3			-3	-3	-3	-3			

	Southlawn Drive Southbound						Beam Avenue Westbound						Southlawn Drive Northbound						Beam Avenue Eastbound							
Time	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Bikes	Right	Thru	Left	U-Turn	Appr Total	Int Total	
AM Pk Hr		12:00	12:00	12:00	12:00	12:00		12:00	12:00	12:00	12:00	12:00		12:00	12:00	12:00	12:00	12:00		12:00	12:00	12:00	12:00	12:00	12:00	12:00
AM Pk Vol		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
AM PHF		NaN	NaN	NaN	NaN	NaN		NaN	NaN	NaN	NaN	NaN		NaN	NaN	NaN	NaN	NaN		NaN	NaN	NaN	NaN	NaN	NaN	NaN
PM Pk Hr		21:15	21:15	21:15	21:15	21:15		21:15	21:15	21:15	21:15	21:15		21:15	21:15	21:15	21:15	21:15		21:15	21:15	21:15	21:15	21:15	21:15	21:15
PM Pk Vol		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	0	0
PM PHF		NaN	NaN	NaN	NaN	NaN		NaN	NaN	NaN	NaN	NaN		NaN	NaN	NaN	NaN	NaN		NaN	NaN	NaN	NaN	NaN	NaN	NaN

# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**All Vehicles**

**Site Code:**  
**Study Date:** 05/17/2022

Time	Southlawn Drive Southbound					Beam Avenue Westbound					Southlawn Drive Northbound					Beam Avenue Eastbound					Int Total
	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	
05:00	0	0	0	0	0	1	2	0	1	4	0	0	1	0	1	0	4	2	0	6	11
05:15	1	0	0	0	1	1	6	0	0	7	0	0	0	0	0	0	3	3	0	6	14
05:30	0	0	1	0	1	1	11	0	0	12	0	0	2	0	2	0	3	3	1	7	22
05:45	1	0	3	0	4	1	16	1	0	18	0	2	3	0	5	1	10	8	0	19	46
<b>Total</b>	<b>2</b>	<b>0</b>	<b>4</b>	<b>0</b>	<b>6</b>	<b>4</b>	<b>35</b>	<b>1</b>	<b>1</b>	<b>41</b>	<b>0</b>	<b>2</b>	<b>6</b>	<b>0</b>	<b>8</b>	<b>1</b>	<b>20</b>	<b>16</b>	<b>1</b>	<b>38</b>	<b>93</b>
06:00	1	2	1	0	4	3	21	5	1	30	1	1	2	0	4	1	7	8	1	17	55
06:15	4	2	1	0	7	5	26	0	0	31	0	1	0	0	1	1	10	9	2	22	61
06:30	4	0	1	0	5	6	36	0	0	42	2	0	2	0	4	5	18	8	0	31	82
06:45	2	2	2	0	6	6	60	3	3	72	3	2	4	0	9	3	46	10	0	59	146
<b>Total</b>	<b>11</b>	<b>6</b>	<b>5</b>	<b>0</b>	<b>22</b>	<b>20</b>	<b>143</b>	<b>8</b>	<b>4</b>	<b>175</b>	<b>6</b>	<b>4</b>	<b>8</b>	<b>0</b>	<b>18</b>	<b>10</b>	<b>81</b>	<b>35</b>	<b>3</b>	<b>129</b>	<b>344</b>
07:00	0	2	5	0	7	8	51	1	0	60	1	0	4	0	5	1	22	7	2	32	104
07:15	4	0	3	0	7	7	59	0	3	69	0	1	2	0	3	0	26	13	4	43	122
07:30	2	0	2	0	4	4	57	2	3	66	2	0	6	0	8	0	55	15	3	73	151
07:45	4	1	6	0	11	7	65	2	2	76	2	0	3	0	5	6	51	14	1	72	164
<b>Total</b>	<b>10</b>	<b>3</b>	<b>16</b>	<b>0</b>	<b>29</b>	<b>26</b>	<b>232</b>	<b>5</b>	<b>8</b>	<b>271</b>	<b>5</b>	<b>1</b>	<b>15</b>	<b>0</b>	<b>21</b>	<b>7</b>	<b>154</b>	<b>49</b>	<b>10</b>	<b>220</b>	<b>541</b>
08:00	3	0	2	0	5	6	71	2	2	81	5	0	4	0	9	3	39	9	1	52	147
08:15	4	4	6	0	14	8	57	3	1	69	5	0	3	0	8	3	76	11	2	92	183
08:30	4	6	8	0	18	9	43	3	3	58	4	4	4	0	12	6	65	14	0	85	173
08:45	6	4	8	0	18	13	42	7	1	63	4	0	7	0	11	8	67	19	1	95	187
<b>Total</b>	<b>17</b>	<b>14</b>	<b>24</b>	<b>0</b>	<b>55</b>	<b>36</b>	<b>213</b>	<b>15</b>	<b>7</b>	<b>271</b>	<b>18</b>	<b>4</b>	<b>18</b>	<b>0</b>	<b>40</b>	<b>20</b>	<b>247</b>	<b>53</b>	<b>4</b>	<b>324</b>	<b>690</b>
09:00	3	3	6	0	12	16	49	3	1	69	6	1	3	0	10	6	67	15	0	88	179
09:15	9	2	10	0	21	11	57	1	2	71	1	3	5	0	9	2	66	19	3	90	191
09:30	8	2	13	0	23	14	70	4	1	89	7	5	7	0	19	3	63	23	2	91	222
09:45	15	1	12	0	28	20	78	6	1	105	5	1	5	0	11	2	90	32	1	125	269
<b>Total</b>	<b>35</b>	<b>8</b>	<b>41</b>	<b>0</b>	<b>84</b>	<b>61</b>	<b>254</b>	<b>14</b>	<b>5</b>	<b>334</b>	<b>19</b>	<b>10</b>	<b>20</b>	<b>0</b>	<b>49</b>	<b>13</b>	<b>286</b>	<b>89</b>	<b>6</b>	<b>394</b>	<b>861</b>

Time	Southlawn Drive Southbound					Beam Avenue Westbound					Southlawn Drive Northbound					Beam Avenue Eastbound					Int Total
	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	
10:00	16	3	19	0	38	19	94	6	4	123	3	0	4	0	7	6	87	32	4	129	297
10:15	17	1	17	0	35	15	71	6	0	92	3	2	4	0	9	2	85	28	2	117	253
10:30	20	2	10	0	32	16	73	5	1	95	9	0	6	0	15	4	84	43	2	133	275
10:45	14	2	10	0	26	24	94	3	3	124	4	0	4	0	8	8	126	27	1	162	320
<b>Total</b>	<b>67</b>	<b>8</b>	<b>56</b>	<b>0</b>	<b>131</b>	<b>74</b>	<b>332</b>	<b>20</b>	<b>8</b>	<b>434</b>	<b>19</b>	<b>2</b>	<b>18</b>	<b>0</b>	<b>39</b>	<b>20</b>	<b>382</b>	<b>130</b>	<b>9</b>	<b>541</b>	<b>1145</b>
11:00	25	4	24	0	53	19	77	9	3	108	4	3	7	0	14	11	92	33	0	136	311
11:15	22	4	26	0	52	19	82	10	0	111	5	7	6	0	18	4	127	41	2	174	355
11:30	26	5	24	0	55	19	74	6	4	103	13	5	8	0	26	6	106	34	1	147	331
11:45	32	5	23	0	60	21	91	5	0	117	4	4	3	0	11	2	134	40	1	177	365
<b>Total</b>	<b>105</b>	<b>18</b>	<b>97</b>	<b>0</b>	<b>220</b>	<b>78</b>	<b>324</b>	<b>30</b>	<b>7</b>	<b>439</b>	<b>26</b>	<b>19</b>	<b>24</b>	<b>0</b>	<b>69</b>	<b>23</b>	<b>459</b>	<b>148</b>	<b>4</b>	<b>634</b>	<b>1362</b>
12:00	23	4	31	0	58	20	88	10	2	120	8	4	7	0	19	5	121	39	0	165	362
12:15	23	2	20	0	45	24	113	10	2	149	8	3	10	0	21	8	108	26	1	143	358
12:30	23	3	24	0	50	21	98	15	0	134	2	5	7	0	14	7	104	39	4	154	352
12:45	18	3	24	0	45	18	101	13	3	135	6	4	9	0	19	5	95	35	5	140	339
<b>Total</b>	<b>87</b>	<b>12</b>	<b>99</b>	<b>0</b>	<b>198</b>	<b>83</b>	<b>400</b>	<b>48</b>	<b>7</b>	<b>538</b>	<b>24</b>	<b>16</b>	<b>33</b>	<b>0</b>	<b>73</b>	<b>25</b>	<b>428</b>	<b>139</b>	<b>10</b>	<b>602</b>	<b>1411</b>
13:00	26	4	33	0	63	20	87	8	1	116	7	2	5	0	14	4	104	39	0	147	340
13:15	19	3	24	0	46	16	120	8	3	147	5	5	2	0	12	3	97	28	2	130	335
13:30	24	5	24	0	53	19	108	12	2	141	7	2	5	0	14	4	112	34	1	151	359
13:45	32	7	33	0	72	17	85	10	5	117	6	5	9	0	20	4	105	33	2	144	353
<b>Total</b>	<b>101</b>	<b>19</b>	<b>114</b>	<b>0</b>	<b>234</b>	<b>72</b>	<b>400</b>	<b>38</b>	<b>11</b>	<b>521</b>	<b>25</b>	<b>14</b>	<b>21</b>	<b>0</b>	<b>60</b>	<b>15</b>	<b>418</b>	<b>134</b>	<b>5</b>	<b>572</b>	<b>1387</b>
14:00	28	3	25	0	56	20	92	6	1	119	11	3	5	0	19	5	107	30	2	144	338
14:15	14	5	22	0	41	27	132	8	1	168	5	2	13	0	20	4	107	29	1	141	370
14:30	28	5	25	0	58	8	93	4	2	107	5	4	11	0	20	8	106	25	3	142	327
14:45	22	2	27	0	51	18	86	6	4	114	7	3	6	0	16	4	97	41	3	145	326
<b>Total</b>	<b>92</b>	<b>15</b>	<b>99</b>	<b>0</b>	<b>206</b>	<b>73</b>	<b>403</b>	<b>24</b>	<b>8</b>	<b>508</b>	<b>28</b>	<b>12</b>	<b>35</b>	<b>0</b>	<b>75</b>	<b>21</b>	<b>417</b>	<b>125</b>	<b>9</b>	<b>572</b>	<b>1361</b>
15:00	19	8	31	0	58	25	90	6	0	121	4	5	12	0	21	7	98	31	1	137	337
15:15	26	3	30	0	59	23	89	6	3	121	4	3	7	0	14	6	117	32	3	158	352
15:30	17	2	21	0	40	20	90	6	4	120	12	2	14	0	28	3	135	34	2	174	362
15:45	19	5	28	0	52	21	74	7	2	104	11	4	9	0	24	9	125	30	2	166	346
<b>Total</b>	<b>81</b>	<b>18</b>	<b>110</b>	<b>0</b>	<b>209</b>	<b>89</b>	<b>343</b>	<b>25</b>	<b>9</b>	<b>466</b>	<b>31</b>	<b>14</b>	<b>42</b>	<b>0</b>	<b>87</b>	<b>25</b>	<b>475</b>	<b>127</b>	<b>8</b>	<b>635</b>	<b>1397</b>
16:00	24	7	24	0	55	17	78	9	1	105	9	1	9	0	19	13	128	46	1	188	367
16:15	24	2	27	1	54	28	87	6	1	122	10	6	12	0	28	6	130	33	1	170	374
16:30	39	5	28	0	72	12	91	9	1	113	10	1	9	0	20	12	137	39	2	190	395
16:45	21	3	20	0	44	19	64	12	0	95	6	2	8	0	16	6	117	40	2	165	320
<b>Total</b>	<b>108</b>	<b>17</b>	<b>99</b>	<b>1</b>	<b>225</b>	<b>76</b>	<b>320</b>	<b>36</b>	<b>3</b>	<b>435</b>	<b>35</b>	<b>10</b>	<b>38</b>	<b>0</b>	<b>83</b>	<b>37</b>	<b>512</b>	<b>158</b>	<b>6</b>	<b>713</b>	<b>1456</b>

Time	Southlawn Drive Southbound					Beam Avenue Westbound					Southlawn Drive Northbound					Beam Avenue Eastbound					Int Total
	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	
17:00	21	4	14	1	40	23	88	7	3	121	6	1	7	0	14	13	133	40	0	186	361
17:15	19	1	19	0	39	16	98	9	1	124	5	3	12	0	20	7	124	45	2	178	361
17:30	22	4	18	0	44	26	78	5	0	109	4	4	10	0	18	16	134	37	1	188	359
17:45	20	8	21	0	49	22	75	14	1	112	7	1	9	0	17	8	105	47	3	163	341
<b>Total</b>	<b>82</b>	<b>17</b>	<b>72</b>	<b>1</b>	<b>172</b>	<b>87</b>	<b>339</b>	<b>35</b>	<b>5</b>	<b>466</b>	<b>22</b>	<b>9</b>	<b>38</b>	<b>0</b>	<b>69</b>	<b>44</b>	<b>496</b>	<b>169</b>	<b>6</b>	<b>715</b>	<b>1422</b>
18:00	30	4	25	0	59	30	91	4	2	127	5	0	8	0	13	5	96	44	2	147	346
18:15	27	2	23	0	52	30	86	11	0	127	3	7	14	0	24	7	89	40	1	137	340
18:30	29	3	25	0	57	23	85	15	1	124	7	3	17	0	27	2	105	56	2	165	373
18:45	26	2	21	0	49	14	71	9	2	96	10	7	6	0	23	7	89	48	0	144	312
<b>Total</b>	<b>112</b>	<b>11</b>	<b>94</b>	<b>0</b>	<b>217</b>	<b>97</b>	<b>333</b>	<b>39</b>	<b>5</b>	<b>474</b>	<b>25</b>	<b>17</b>	<b>45</b>	<b>0</b>	<b>87</b>	<b>21</b>	<b>379</b>	<b>188</b>	<b>5</b>	<b>593</b>	<b>1371</b>
19:00	25	3	29	0	57	16	59	7	2	84	5	4	7	0	16	2	66	37	1	106	263
19:15	34	3	25	0	62	13	70	6	2	91	4	7	6	0	17	7	48	17	5	77	247
19:30	22	1	15	0	38	18	78	4	3	103	3	0	6	0	9	5	67	27	1	100	250
19:45	23	1	21	0	45	14	80	6	4	104	7	0	15	0	22	5	56	22	0	83	254
<b>Total</b>	<b>104</b>	<b>8</b>	<b>90</b>	<b>0</b>	<b>202</b>	<b>61</b>	<b>287</b>	<b>23</b>	<b>11</b>	<b>382</b>	<b>19</b>	<b>11</b>	<b>34</b>	<b>0</b>	<b>64</b>	<b>19</b>	<b>237</b>	<b>103</b>	<b>7</b>	<b>366</b>	<b>1014</b>
20:00	26	2	21	0	49	17	61	9	6	93	5	2	8	0	15	6	54	16	2	78	235
20:15	22	3	19	1	45	7	41	6	3	57	7	0	6	0	13	8	55	6	2	71	186
20:30	12	0	16	0	28	7	37	2	1	47	2	2	9	0	13	4	38	10	0	52	140
20:45	8	0	7	0	15	4	24	4	2	34	3	2	6	0	11	2	27	12	0	41	101
<b>Total</b>	<b>68</b>	<b>5</b>	<b>63</b>	<b>1</b>	<b>137</b>	<b>35</b>	<b>163</b>	<b>21</b>	<b>12</b>	<b>231</b>	<b>17</b>	<b>6</b>	<b>29</b>	<b>0</b>	<b>52</b>	<b>20</b>	<b>174</b>	<b>44</b>	<b>4</b>	<b>242</b>	<b>662</b>
21:00	10	1	8	0	19	6	39	3	0	48	1	0	2	0	3	1	34	3	0	38	108
21:15	13	0	8	0	21	3	28	0	2	33	1	0	1	0	2	1	22	4	0	27	83
21:30	9	2	3	0	14	0	18	5	0	23	12	3	5	0	20	2	17	5	0	24	81
21:45	11	0	8	0	19	1	13	0	0	14	3	0	4	0	7	2	20	5	2	29	69
<b>Total</b>	<b>43</b>	<b>3</b>	<b>27</b>	<b>0</b>	<b>73</b>	<b>10</b>	<b>98</b>	<b>8</b>	<b>2</b>	<b>118</b>	<b>17</b>	<b>3</b>	<b>12</b>	<b>0</b>	<b>32</b>	<b>6</b>	<b>93</b>	<b>17</b>	<b>2</b>	<b>118</b>	<b>341</b>
<b>Grand Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>Appr %</b>	<b>46.5</b>	<b>07.5</b>	<b>45.9</b>	<b>00.1</b>		<b>16.1</b>	<b>75.7</b>	<b>06.4</b>	<b>01.9</b>		<b>36.3</b>	<b>16.6</b>	<b>47.1</b>	<b>00.0</b>		<b>04.4</b>	<b>71.0</b>	<b>23.3</b>	<b>01.3</b>		
<b>Total %</b>	<b>06.7</b>	<b>01.1</b>	<b>06.6</b>	<b>00.0</b>		<b>05.8</b>	<b>27.4</b>	<b>02.3</b>	<b>00.7</b>		<b>02.0</b>	<b>00.9</b>	<b>02.6</b>	<b>00.0</b>		<b>01.9</b>	<b>31.2</b>	<b>10.2</b>	<b>00.6</b>		
<b>% Trucks</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>-</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>	<b>00.0</b>

	Southlawn Drive Southbound					Beam Avenue Westbound					Southlawn Drive Northbound					Beam Avenue Eastbound						
Time	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Right	Thru	Left	U-Turn	Appr Total	Int Total	
AM Pk Hr	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45	11:45
AM Pk Vol	101	14	98	0	213	86	390	40	4	520	22	16	27	0	65	22	467	144	6	639	1437	
AM PHF	0.789	0.700	0.790	NaN	0.888	0.896	0.863	0.667	0.500	0.872	0.688	0.800	0.675	NaN	0.774	0.688	0.871	0.900	0.375	0.903	0.984	
PM Pk Hr	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45	15:45
PM Pk Vol	106	19	107	1	233	78	330	31	5	444	40	12	39	0	91	40	520	148	6	714	1482	
PM PHF	0.679	0.679	0.955	0.250	0.809	0.696	0.907	0.861	0.625	0.910	0.909	0.500	0.813	NaN	0.813	0.769	0.949	0.804	0.750	0.939	0.938	

# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Site Code:**  
**Study Date:** 05/17/2022

**All Vehicles**

Beam Avenue		In = 6279		Out = 7408	
	327	5258	1724	99	
	Right	Thru	Left	U-Turn	
					

Southlawn Drive			
In = 2863		Out = 2420	
1125	182	1110	3
Right	Thru	Left	U-Turn
			

Total Volumes  
 05:00 to 22:00  
 Volume = 16858

Beam Avenue		In = 6817		Out = 6104	
	982	4619	390	113	
	Right	Thru	Left	U-Turn	
					

			
U-Turn	Left	Thru	Right
0	436	154	336
In = 899		Out = 926	
Southlawn Drive			

# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Site Code:**  
**Study Date:** 05/17/2022

**All Vehicles**

Beam Avenue	6	U-Turn	
	144	Left	
	467	Thru	
	22	Right	

Southlawn Drive			
101	14	98	0
Right	Thru	Left	U-Turn
			

**AM Peak Hour Statistics**  
 AM Peak Hour Begins: 11:45  
 AM Peak Hour Volume: 1437  
 AM Peak Hour Factor: 0.984

86	Right	Beam Avenue
390	Thru	
40	Left	
4	U-Turn	
		
		
		
		

			
U-Turn	Left	Thru	Right
0	27	16	22
Southlawn Drive			

# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Site Code:**  
**Study Date:** 05/17/2022

**All Vehicles**

Beam Avenue	6	U-Turn	
	148	Left	
	520	Thru	
	40	Right	

Southlawn Drive			
106	19	107	1
Right	Thru	Left	U-Turn
			

**PM Peak Hour Statistics**  
 PM Peak Hour Begins: 15:45  
 PM Peak Hour Volume: 1482  
 PM Peak Hour Factor: 0.938

78	Right	Beam Avenue
330	Thru	
31	Left	
5	U-Turn	
		
		
		
		

			
U-Turn	Left	Thru	Right
0	39	12	40
Southlawn Drive			

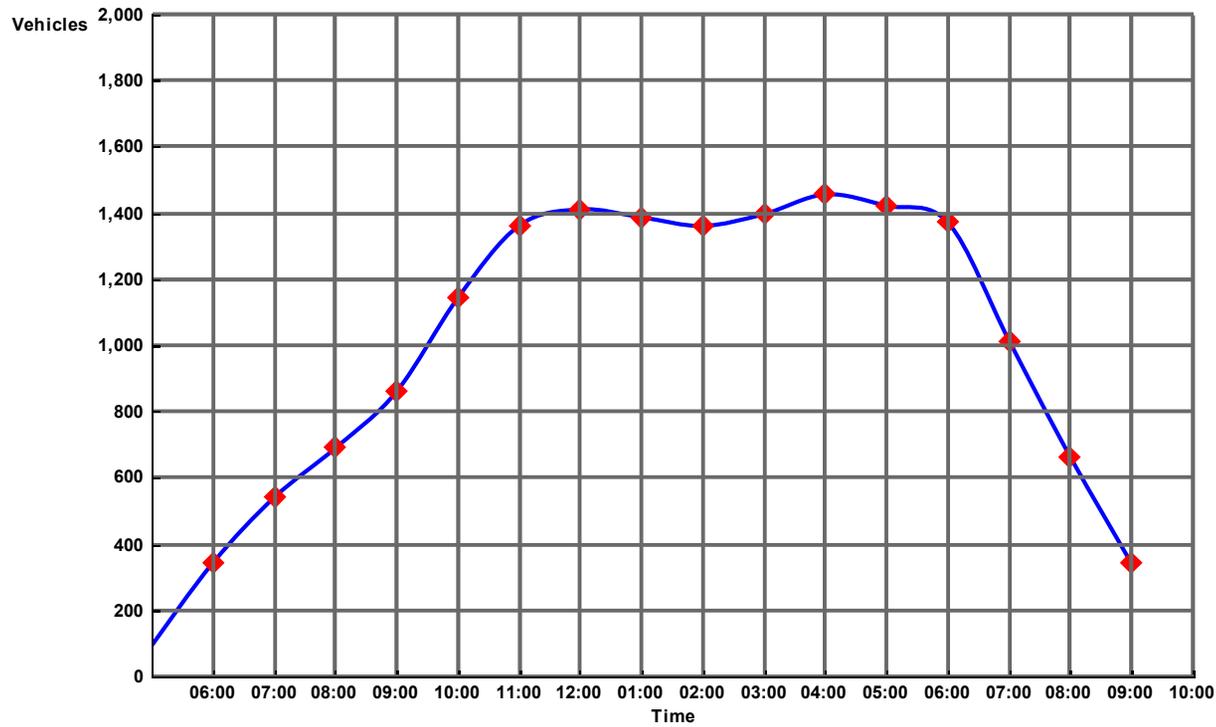
# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Site Code:**  
**Study Date:** 05/17/2022

Cars



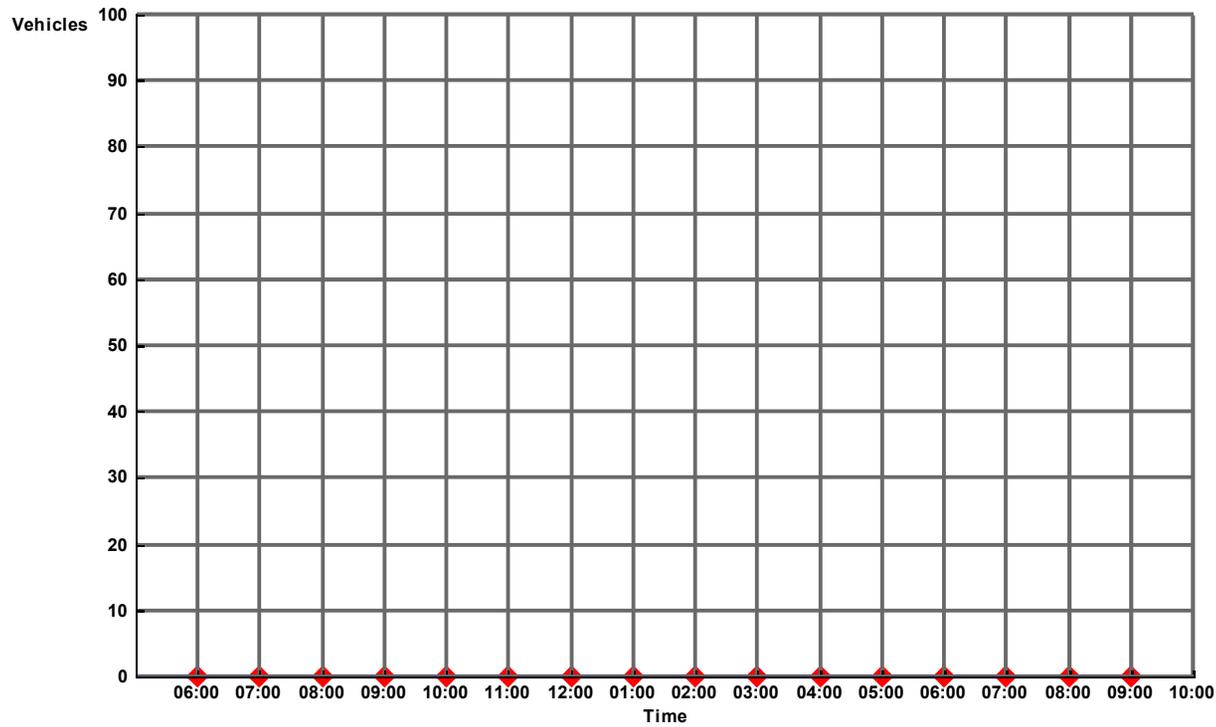
# Ramsey County Public Works

## Turning Movement Count

**File Name:** Beam Avenue at Southlawn Drive 0500-2200  
**Location:** Beam Avenue at Southlawn Drive

**Site Code:**  
**Study Date:** 05/17/2022

Trucks



## **APPENDIX B**

# **Information from the Minnesota Department of Transportation (MnDOT) Traffic Mapping Application Growth Rate Calculations**

Find address or place

+

-

Home icon

Refresh icon



SEQUENCE_NUMBER	DATA_YEAR	AADT	DATA_TYPE	AADT_COMMENTS	ROUTE_LABEL	STREET_NAME	LOCATION_DESCRIPTION	COUNTY_NAME	CITY_NAME
37586	2021	2485	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	2018	4300	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	2017	4450	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	2013	4750	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	2009	4850	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	2005	3400	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	2001	3400	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37586	1997	2600	A	21 COVID, 17 Avg Hist (13 Cnt), 09 RCH, New Dev	MSAS 117	Southlawn Dr	S OF MSAS121 (CO RD D)	Ramsey	Maplewood
37587	2020	11600	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	
37587	2014	9500	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	
37587	2011	13000	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	
37587	2009	13000	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	
37587	2005	13000	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	
37587	2001	11800	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	
37587	1997	15000	A	20 COVID, 18 No AADT, 14 See Hist, 13 No AADT, 11 E	CSAH 19	E County Rd D	W OF CSAH65 (WHITE BEAR AV)	Ramsey	

# Westwood GROWTH RATE CALCULATIONS

Project Name: Enclave - Maplewood  
 Calculations by: PJV Date: September 16, 2022 Project #: R0037066.00

## Minnesota Department of Transportation - 2022 Traffic Mapping Application

*This spreadsheet calculates average annual growth at the identified MnDOT count stations and then averages those growth rates to estimate the average annual growth rate for the study area.*

### 0.8% Study Area Average Annual Growth Rate (Average of all MnDOT Count Stations)

#### 2 Number of MnDOT count stations

ID: 1		MnDOT Count Station: 37586	
Roadway: Southlawn Dr			
Location: S of MSAS121 (CO RD D)			
Year	ADT	Average Annual Growth	-1.9%
2013	4,750		
2018	4,322	Projected Traffic Volumes Using Average Annual Growth Rate	
2020	4,162		
2023	3,933		
2024	3,859		

ID: 2		MnDOT Count Station: 37587	
Roadway: E County Rd D			
Location: W of CSAH65 (WHITE BEAR AV)			
Year	ADT	Average Annual Growth	3.4%
2014	9,500		
2020	11,600	Projected Traffic Volumes Using Average Annual Growth Rate	
2022	12,399		
2025	13,701		
2026	14,164		

ID:		MnDOT Count Station:	
Roadway:			
Location:			
Year	ADT	Average Annual Growth	
		Projected Traffic Volumes Using Average Annual Growth Rate	

ID: 4		MnDOT Count Station:	
Roadway:			
Location:			
Year	ADT	Average Annual Growth	
		Projected Traffic Volumes Using Average Annual Growth Rate	

ID: 5		MnDOT Count Station:	
Roadway:			
Location:			
Year	ADT	Average Annual Growth	
		Projected Traffic Volumes Using Average Annual Growth Rate	

ID: 6		MnDOT Count Station:	
Roadway:			
Location:			
Year	ADT	Average Annual Growth	
		Projected Traffic Volumes Using Average Annual Growth Rate	

ID: 7		MnDOT Count Station:	
Roadway:			
Location:			
Year	ADT	Average Annual Growth	
		Projected Traffic Volumes Using Average Annual Growth Rate	

ID:		MnDOT Count Station:	
Roadway:			
Location:			
Year	ADT	Average Annual Growth	
		Projected Traffic Volumes Using Average Annual Growth Rate	

# APPENDIX C

## Project Trip Generation



# TRIP GENERATION CALCULATIONS

Project Name: Enclave - Myth  
 Calculations by: PJV Date: September 16, 2022 Project #: R0037066.00

## ITE Trip Generation - 11th Edition

Equation Type:	Average Rate
Land Use Code:	221
Variable (X):	Dwelling Units

## Multifamily Housing (Mid-Rise)

<b>Number of (X):</b>	<b>241</b>
-----------------------	------------

T = Average Vehicle Trip Ends

### AM Peak Hour

One hour between 7AM to 9AM (Adjacent Street)

T = (X) \* 0.37      Trip Ends Per Dwelling Units  
 T = 89      Trip Ends

Directional Distribution:			
23%	Entering	77%	Exiting
<u>21</u>	Entering*	<u>69</u>	Exiting*

*\*Rounding may occur in calculations*

### PM Peak Hour

One hour between 4PM to 6PM (Adjacent Street)

T = (X) \* 0.39      Trip Ends Per Dwelling Units  
 T = 94      Trip Ends

Directional Distribution:			
61%	Entering*	39%	Exiting
<u>57</u>	Entering*	<u>37</u>	Exiting*

*\*Rounding may occur in calculations*

### Weekday

Daily Weekday

T = (X) \* 4.54      Trip Ends Per Dwelling Units  
 T = 1,094      Trip Ends

Directional Distribution:			
50%	Entering	50%	Exiting
<u>547</u>	Entering*	<u>547</u>	Exiting*

*\*Rounding may occur in calculations*

### Pass-By Reduction

Non Pass-By Trip End Percentage	
AM	100%
PM	100%

Pass-By Trip End Percentage	
AM	0%
PM	0%

Non Pass-By Trip Ends:

Pass-By Trip Ends:

<b>AM</b>	<u>21</u>	Entering	<u>69</u>	Exiting
<b>PM</b>	<u>57</u>	Entering	<u>37</u>	Exiting

<b>AM</b>	<u>0</u>	Entering	<u>0</u>	Exiting
<b>PM</b>	<u>0</u>	Entering	<u>0</u>	Exiting

*\*Rounding may occur in calculations*

*\*Rounding may occur in calculations*

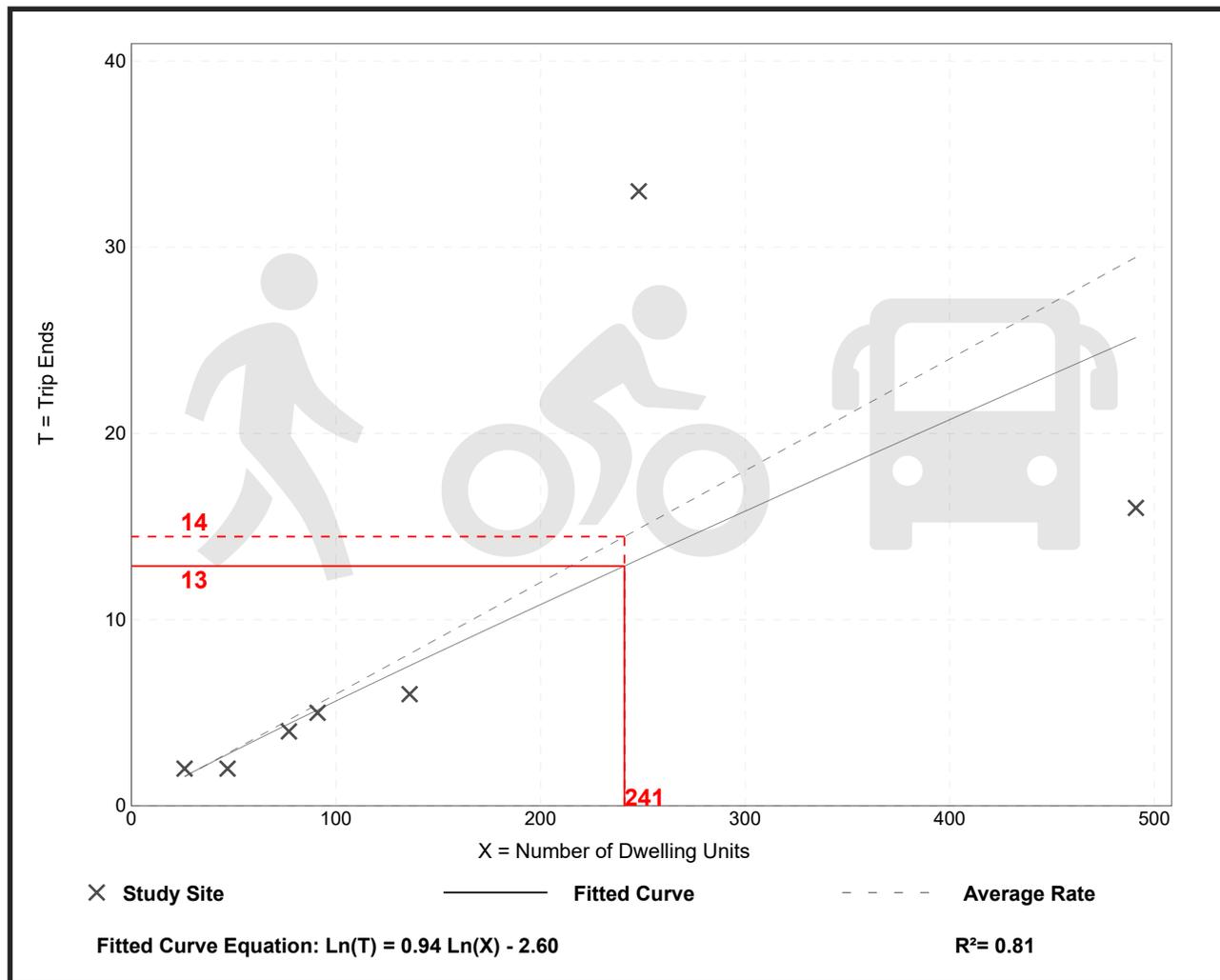
# Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

**Walk+Bike+Transit Trip Ends vs: Dwelling Units**  
**On a: Weekday,**  
**Peak Hour of Adjacent Street Traffic,**  
**One Hour Between 7 and 9 a.m.**  
**Setting/Location: General Urban/Suburban**  
 Number of Studies: 7  
 Avg. Num. of Dwelling Units: 159  
 Directional Distribution: 27% entering, 73% exiting

## Walk+Bike+Transit Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.06	0.03 - 0.13	0.04

## Data Plot and Equation



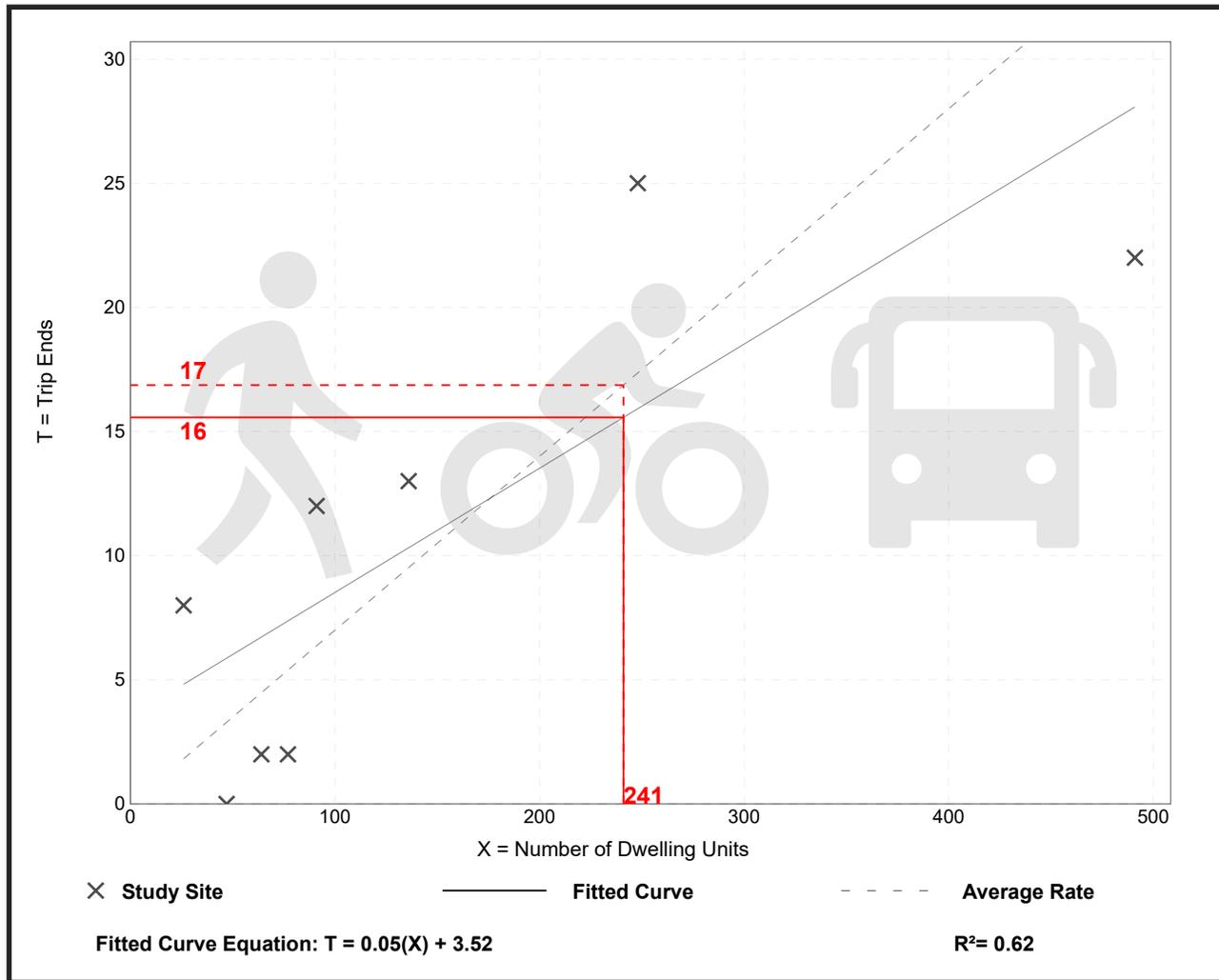
# Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Walk+Bike+Transit Trip Ends vs: Dwelling Units  
 On a: Weekday,  
 Peak Hour of Adjacent Street Traffic,  
 One Hour Between 4 and 6 p.m.  
 Setting/Location: General Urban/Suburban  
 Number of Studies: 8  
 Avg. Num. of Dwelling Units: 148  
 Directional Distribution: 55% entering, 45% exiting

## Walk+Bike+Transit Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.07	0.00 - 0.31	0.05

## Data Plot and Equation



## **APPENDIX D**

### **Traffic Signal Timing Plan at County Road D East / Southlawn Drive**

### **Traffic Signal Plan at County Road D East / Southlawn Drive**

### **VISTRO Capacity Analyses**

**NOTES:**

- THE LOCATION OF POLES, LOOP DETECTORS, VIDEO DETECTION CAMERAS, AND EQUIPMENT PAD SHALL BE APPROVED BY THE ENGINEER PRIOR TO INSTALLATION.
- ITEMS DENOTED WITH A ● ARE INCIDENTAL TO THE INTERCONNECT PAY ITEM.
- SIGNING SHOWN IN POLE NOTES ARE INCIDENTAL TO THE SIGNAL SYSTEM.
- ALL MAST ARM MOUNTED TYPE "D" SIGNS ARE INCIDENTAL TO THE SIGNAL SYSTEM.
- CONTRACTOR SHALL VERIFY GROUND WATER LEVELS WHEN INSTALLING POLE, FOUNDATIONS AND SHALL DE-WATER AS NECESSARY DURING FOUNDATION INSTALLATION. THIS WORK SHALL BE INCIDENTAL TO THE SIGNAL SYSTEM.
- NO OPEN CUT TRENCHING OF THE ROADWAYS WILL BE ALLOWED IN AREAS THAT ARE OPEN TO TRAFFIC OR WILL BE REOPENED TO TRAFFIC PRIOR TO THE INSTALLATION OF PAVEMENT.
- SEE OTHER PLAN SHEETS FOR PEDESTRIAN RAMPS, GROUND MOUNTED SIGNING, PAVEMENT AND CROSSWALK MARKINGS, AND ROADWAY PLANS.
- THE LOCATION OF THE TRANSFORMER HAS NOT BEEN FINALIZED. THE CONTRACTOR SHALL COORDINATE WITH XCEL ENERGY.
- VIDEO DETECTION CAMERAS SHALL BE MOUNTED ON 6 FT. HEIGHT EXTENSION BRACKET.
- ALL PEDESTRIAN DETECTORS SHALL BE ACCESSIBLE PEDESTRIAN DETECTORS. PEDESTRIAN PUSH BUTTON STATION SHALL BE IN ACCORDANCE WITH THE DETAILS. SEE PEDESTRIAN PUSH BUTTON STATION DETAIL.
- INTERCONNECT, LISTED AS 1-24/C SM/MM FIBER, SHALL BE 12 MULTIMODE, 12 SINGLEMODE FIBER OPTIC CABLE FROM MN/DOT APPROVED PRODUCT LIST.

**(A) EQUIPMENT PAD - SEE DETAILS**

- SERVICE CABINET EXTEND INTO HH 8
- 2" NMC
- 3-1/C #4
- EXTEND INTO HH 1
- 2" NMC
- 1-3/C #14 (LUM)
- EXTEND INTO HH 7
- 2" NMC
- 1-3/C #14 (LUM)

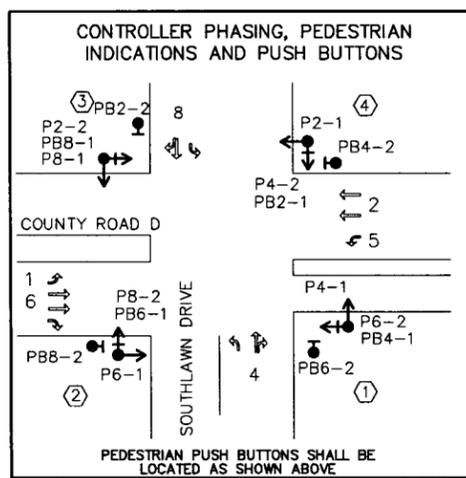
- CONTROLLER CABINET EXTEND INTO HH 1
- 4" NMC
- 4-12/C #14
- 3-3/C #14
- 2-3/C #18
- 2-3/C #20
- 2-1/C #6 INS. GRD.
- 4-2/C #14
- EXTEND INTO HH 7
- 4" NMC
- 4-12/C #14
- 3-3/C #14
- 2-3/C #18
- 2-3/C #20
- 2-1/C #6 INS. GRD.
- 4-2/C #14

- 1-24/C SM/MM FIBER
- 1-1/C #14

- BETWEEN SERVICE AND CONTROLLER CABINET
- 1.25" NMC
- 2-1/C #6
- 1-1/C #6 (BARE)

**(B) SOP - TRANSFORMER (BY XCEL)**

- 2' CONDUIT TO HH 6
- 3-1/C #4
- 2" WEATHERHEAD AND RISER

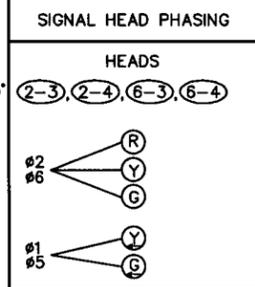


**SIGNAL SYSTEM OPERATIONS**

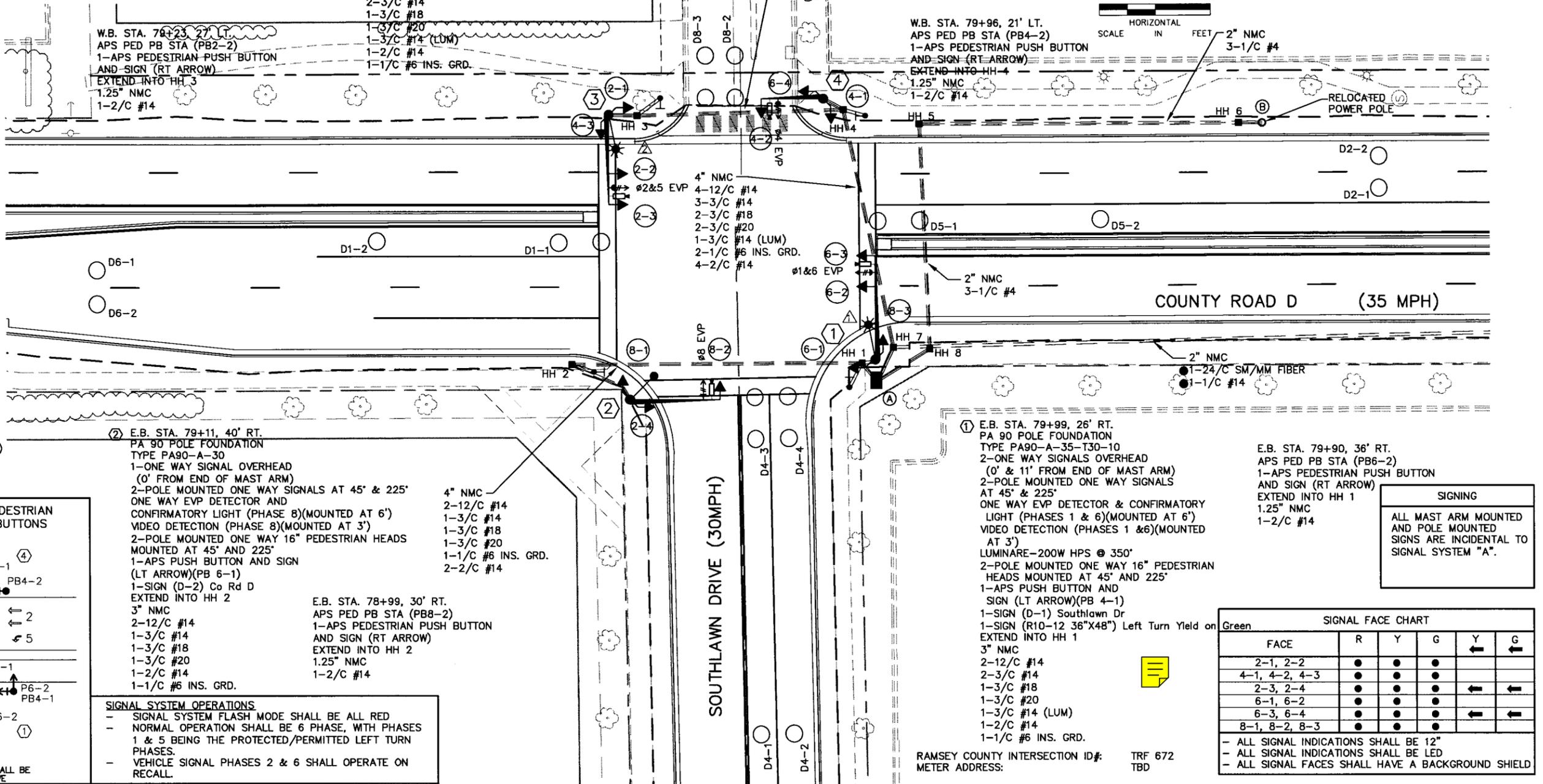
- SIGNAL SYSTEM FLASH MODE SHALL BE ALL RED
- NORMAL OPERATION SHALL BE 6 PHASE, WITH PHASES 1 & 5 BEING THE PROTECTED/PERMITTED LEFT TURN PHASES.
- VEHICLE SIGNAL PHASES 2 & 6 SHALL OPERATE ON RECALL.

- (3) W.B. STA. 79+04, 21' LT.**  
 PA 90 POLE FOUNDATION  
 TYPE PA85-A-30-T30-10  
 2-ONE WAY SIGNALS OVERHEAD  
 (0' & 11' FROM END OF MAST ARM)  
 2-POLE MOUNTED ONE WAY SIGNALS AT 45° AND 225°  
 ONE WAY EVP DETECTOR & CONFIRMATORY LIGHT (PHASES 2 & 5)(MOUNTED AT 6')  
 VIDEO DETECTION (PHASES 2 & 5)(MOUNTED AT 3')  
 LUMINARE-200W HPS @ 350'  
 2-POLE MOUNTED ONE WAY  
 16" PEDESTRIAN HEADS MOUNTED AT 45° AND 225°  
 1-APS PUSH BUTTON AND SIGN (LT ARROW)(PB8-1)  
 1-SIGN (D-3) Southlawn Dr  
 1-SIGN (R10-12, 36"X48") Left Turn Yield on Green  
 EXTEND INTO HH 3  
 3" NMC  
 2-12/C #14  
 2-3/C #14  
 1-3/C #18  
 1-3/C #20  
 1-3/C #14 (LUM)  
 1-1/C #6 INS. GRD.  
 2-2/C #14

- (4) W.B. STA. 79+81, 27' LT.**  
 PA 85 POLE FOUNDATION  
 TYPE PA85-A-20  
 1-ONE WAY SIGNAL OVERHEAD  
 (0' FROM END OF MAST ARM)  
 2-POLE MOUNTED ONE WAY SIGNALS AT 45° & 225°  
 ONE WAY EVP DETECTOR & CONFIRMATORY LIGHT (PHASE 4)(MOUNTED AT 6')  
 VIDEO DETECTION (PHASE 4)(MOUNTED AT 3')  
 2-POLE MOUNTED ONE WAY  
 16" PEDESTRIAN HEADS MOUNTED AT 45° AND 225°  
 1-APS PUSH BUTTON AND SIGN (LT ARROW)(PB2-1)  
 1-SIGN (D-4) Co Rd D  
 EXTEND INTO HH 4  
 3" NMC  
 2-12/C #14  
 1-3/C #14  
 1-3/C #18  
 1-3/C #20  
 1-2/C #14  
 1-1/C #6 INS. GRD.



DETECTORS		
NUMBER	TYPE	LOCATION
D1-1	VIDEO	0' & 15'
D1-2	VIDEO	80'
D2-1	VIDEO	180'
D2-2	VIDEO	180'
D4-1	VIDEO	120'
D4-2	VIDEO	120'
D4-3	VIDEO	0' & 15'
D4-4	VIDEO	0' & 15'
D5-1	VIDEO	0' & 15'
D5-2	VIDEO	80'
D6-1	VIDEO	180'
D6-2	VIDEO	180'
D8-1	VIDEO	80'
D8-2	VIDEO	0' & 15'
D8-3	VIDEO	0' & 15'



- (1) E.B. STA. 79+99, 26' RT.**  
 PA 90 POLE FOUNDATION  
 TYPE PA90-A-35-T30-10  
 2-ONE WAY SIGNALS OVERHEAD  
 (0' & 11' FROM END OF MAST ARM)  
 2-POLE MOUNTED ONE WAY SIGNALS AT 45° & 225°  
 ONE WAY EVP DETECTOR & CONFIRMATORY LIGHT (PHASES 1 & 6)(MOUNTED AT 6')  
 VIDEO DETECTION (PHASES 1 & 6)(MOUNTED AT 3')  
 LUMINARE-200W HPS @ 350'  
 2-POLE MOUNTED ONE WAY 16" PEDESTRIAN HEADS MOUNTED AT 45° AND 225°  
 1-APS PUSH BUTTON AND SIGN (LT ARROW)(PB 4-1)  
 1-SIGN (D-1) Southlawn Dr  
 1-SIGN (R10-12 36"X48") Left Turn Yield on Green  
 EXTEND INTO HH 1  
 3" NMC  
 2-12/C #14  
 2-3/C #14  
 1-3/C #18  
 1-3/C #20  
 1-3/C #14 (LUM)  
 1-2/C #14  
 1-1/C #6 INS. GRD.

- E.B. STA. 79+90, 36' RT.**  
 APS PED PB STA (PB6-2)  
 1-APS PEDESTRIAN PUSH BUTTON AND SIGN (RT ARROW)  
 EXTEND INTO HH 1  
 1.25" NMC  
 1-2/C #14

**SIGNING**

ALL MAST ARM MOUNTED AND POLE MOUNTED SIGNS ARE INCIDENTAL TO SIGNAL SYSTEM "A".

SIGNAL FACE CHART						
FACE	R	Y	G	Y	G	
2-1, 2-2	●	●	●	←	←	
4-1, 4-2, 4-3	●	●	●			
2-3, 2-4	●	●	●	←	←	
6-1, 6-2	●	●	●			
6-3, 6-4	●	●	●	←	←	
8-1, 8-2, 8-3	●	●	●			

- ALL SIGNAL INDICATIONS SHALL BE 12"
- ALL SIGNAL INDICATIONS SHALL BE LED
- ALL SIGNAL FACES SHALL HAVE A BACKGROUND SHIELD

RAMSEY COUNTY INTERSECTION ID#: TRF 672  
 METER ADDRESS: TBD

K:\TMC\Civil\County\RAMSEY\WHITE\BEAR\DWG\PLAN\_SHEETS\COUNTY\\_ROAD\\_D\\_CRD-TSIG05.dwg, February 06, 2013 - 2:21pm

No.	Date	Revisions	App.

DRAWING NAME	CRD-TSIG05.dwg
DESIGNED BY:	BJB
DRAWN BY:	HMW
CHECKED BY:	BJB
DATE:	08/07/09
PROJECT NO.	160500029.2.101



I HEREBY CERTIFY THAT THIS PLAN, SPECIFICATION OR REPORT WAS PREPARED BY ME OR UNDER MY DIRECT SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF MINNESOTA.

BRANDON J. BOURDON  
 DATE: 08/07/09 MN LIC. NO. 43709



RAMSEY COUNTY  
 CITY OF MAPLEWOOD  
 COUNTY ROAD D  
 IMPROVEMENTS

TRAFFIC SIGNALS (SYSTEM "A")  
 INTERSECTION LAYOUT  
 COUNTY ROAD D/SOUTHLAWN DRIVE

CITY PROJECT	08-13	SHEET NO. <b>53</b>
COUNTY PROJECT	P-3202	
S.P.	62-619-30	<b>104</b>
S.P.	62-596-04	

Ramsey County, MN



MOVING TRAFFIC FORWARD

Co Rd D @ Southlawn Dr - 672 - Econolite Type - ASC/3

Controller Timing Plan (MM) 2-1

Plan 1

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction																
Min Green	5	15	0	7	5	15	0	7	0	0	0	0	0	0	0	0
Bk Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	7	0	7	0	7	0	7	0	0	0	0	0	0	0	0
Walk2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	11	0	20	0	20	0	23	0	0	0	0	0	0	0	0
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	2.0	3.5	0.0	2.7	2.0	3.5	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max1	15	60	0	15	15	60	0	15	0	0	0	0	0	0	0	0
Max2	0	0	0	35	0	0	0	35	0	0	0	0	0	0	0	0
Max3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Dym Step	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	3.0	4.0	0.0	3.0	3.0	4.0	0.0	3.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Clear	2.0	1.0	0.0	2.2	2.0	1.4	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Act B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Sec/Act	0.0	1.5	0.0	0.0	0.0	1.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	0	18	0	0	0	18	0	0	0	0	0	0	0	0	0	0
Time B4	0	20	0	0	0	20	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPTDuc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
TTReduc	0	20	0	0	0	20	0	0	0	0	0	0	0	0	0	0
Min Gap	2.0	3.0	0.0	2.7	2.0	3.0	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Ramsey County, MN



MOVING TRAFFIC FORWARD

Co Rd D @ Southlawn Dr - 672 - Econolite Type - ASC/3

**Coordination Options  
Options (MM) 3-1**

Manual Pattern	Auto	ECPI Coord	Yes
System Source	SYS	System Format	PTN
Splits In	Seconds	Offsets In	Seconds
Transition	Smooth	Max Select	MAXINH
Dwell / Add Time	0		
Delay Coord Wk-LZ	No	Force Off	Fixed
Offset Reference	Lead	Use Ped Time	No
Ped Recall	No	Ped Reservice	No
Local Zero Override	No	FO Added Ini Green	Yes
Re-sync Count	3	Multisync	No

**Auto Perm Minimum Green (Seconds) (MM) 3-4**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Minimum Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

**Split Demand (MM) 3-5**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Demand 1																
Demand 2																

Demand	1	2
Detector	0	0
Call Time (Sec)	0	0
Cycle Count	0	0

Ramsey County, MN



MOVING TRAFFIC FORWARD

Co Rd D @ Southlawn Dr - 672 - Econolite Type - ASC/3

**Coordination Pattern Data**  
**Coordinator Pattern Data (MM) 3-2**

**Coordinator Pattern # 2**

Split Pattern	2	TS2 (Pat-Off)	0-2	Splits In	Seconds
Cycle	55	Std (COS)	11	Offsets In	Seconds
Offset Value	22s	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	1		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	2		
Max Select	None	Force Off	None		

**Split Preference Phases**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Splits (Split Pat 2)	11	27	0	17	11	27	0	17	0	0	0	0	0	0	0	0
Pref 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pref 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Displacement	-	0	0	0
Split Sum	55s	55s	0s	0s

Misc. Data			
Veh Perm 1	0	Veh Perm 2	0
Veh Perm 2 Disp	0	Split Demand Pat 1	0
Split Demand Pat 2	0	Split Demand Pat 2	0
Crossing Arterial Pat	0		

**Split Pattern**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																
Special Function Outputs																

**Coordinator Pattern # 4**

Split Pattern	4	TS2 (Pat-Off)	1-1	Splits In	Seconds
Cycle	55	Std (COS)	156	Offsets In	Seconds
Offset Value	53s	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	1		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	4		
Max Select	None	Force Off	None		

**Split Preference Phases**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Splits (Split Pat 4)	11	27	0	17	11	27	0	17	0	0	0	0	0	0	0	0
Pref 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pref 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Displacement	-	0	0	0
Split Sum	55s	55s	0s	0s

Misc. Data			
Veh Perm 1	0	Veh Perm 2	0
		Veh Perm 2 Disp	0
Split Demand Pat 1	0	Split Demand Pat 2	0
		Crossing Arterial Pat	0

**Split Pattern**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																
Special Function Outputs																

**Coordinator Pattern # 5**

Split Pattern	5	TS2 (Pat-Off)	1-2	Splits In	Seconds
Cycle	60	Std (COS)	84	Offsets In	Seconds
Offset Value	54s	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	1		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	5		
Max Select	None	Force Off	None		

**Split Preference Phases**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Splits (Split Pat 5)	12	30	0	18	13	29	0	18	0	0	0	0	0	0	0	0
Pref 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pref 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Displacement	-	0	0	0
Split Sum	60s	60s	0s	0s

Misc. Data			
Veh Perm 1	0	Veh Perm 2	0
		Veh Perm 2 Disp	0
Split Demand Pat 1	0	Split Demand Pat 2	0
		Crossing Arterial Pat	0

**Split Pattern**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																
Special Function Outputs																

**Coordinator Pattern # 6**

Split Pattern	6	TS2 (Pat-Off)	1-3	Splits In	Seconds
Cycle	60	Std (COS)	85	Offsets In	Seconds
Offset Value	38s	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	1		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	6		
Max Select	None	Force Off	None		

**Split Preference Phases**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Splits (Split Pat 6)	12	30	0	18	13	29	0	18	0	0	0	0	0	0	0	0
Pref 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pref 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Displacement	-	0	0	0
Split Sum	60s	60s	0s	0s

Misc. Data			
Veh Perm 1	0	Veh Perm 2	0
		Veh Perm 2 Disp	0
Split Demand Pat 1	0	Split Demand Pat 2	0
		Crossing Arterial Pat	0

**Split Pattern**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																
Special Function Outputs																

**Coordinator Pattern # 8**

Split Pattern	8	TS2 (Pat-Off)	2-2	Splits In	Seconds
Cycle	80	Std (COS)	158	Offsets In	Seconds
Offset Value	7s	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	1		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	8		
Max Select	None	Force Off	None		

**Split Preference Phases**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Splits (Split Pat 8)	12	40	0	28	17	35	0	28	0	0	0	0	0	0	0	0
Pref 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pref 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Displacement	-	0	0	0
Split Sum	80s	80s	0s	0s

Misc. Data			
Veh Perm 1	0	Veh Perm 2	0
		Veh Perm 2 Disp	0
Split Demand Pat 1	0	Split Demand Pat 2	0
		Crossing Arterial Pat	0

**Split Pattern**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																
Special Function Outputs																

**Coordinator Pattern # 9**

Split Pattern	9	TS2 (Pat-Off)	2-3	Splits In	Seconds
Cycle	70	Std (COS)	155	Offsets In	Seconds
Offset Value	66s	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	1		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	9		
Max Select	None	Force Off	None		

**Split Preference Phases**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Splits (Split Pat 9)	15	30	0	25	15	30	0	25	0	0	0	0	0	0	0	0
Pref 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pref 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Displacement	-	0	0	0
Split Sum	70s	70s	0s	0s

Misc. Data			
Veh Perm 1	0	Veh Perm 2	0
		Veh Perm 2 Disp	0
Split Demand Pat 1	0	Split Demand Pat 2	0
		Crossing Arterial Pat	0

**Split Pattern**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																
Special Function Outputs																

Ramsey County, MN



MOVING TRAFFIC FORWARD

Co Rd D @ Southlawn Dr - 672 - Econolite Type - ASC/3

**Coordination Split Pattern**  
**Split Pattern Data (MM) 3-3**

**Split Pattern # 2**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Split (seconds)	11	27	0	17	11	27	0	17	0	0	0	0	0	0	0	0
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																

Ring	1	2	3	4
Split Sum	55s	55s	0s	0s

**Split Pattern # 4**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Split (seconds)	11	27	0	17	11	27	0	17	0	0	0	0	0	0	0	0
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																

Ring	1	2	3	4
Split Sum	55s	55s	0s	0s

**Split Pattern # 5**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																
Split (seconds)	12	30	0	18	13	29	0	18	0	0	0	0	0	0	0	0
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																

Ring	1	2	3	4
Split Sum	60s	60s	0s	0s

**Split Pattern # 6**

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description																

Split (seconds)	12	30	0	18	13	29	0	18	0	0	0	0	0	0	0	0
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																

<b>Ring</b>	<b>1</b>	<b>2</b>	<b>3</b>	<b>4</b>
Split Sum	60s	60s	0s	0s

**Split Pattern # 8**

<b>Phase</b>	<b>1</b>	<b>2</b>	<b>3</b>	<b>4</b>	<b>5</b>	<b>6</b>	<b>7</b>	<b>8</b>	<b>9</b>	<b>10</b>	<b>11</b>	<b>12</b>	<b>13</b>	<b>14</b>	<b>15</b>	<b>16</b>
Description																
Split (seconds)	12	40	0	28	17	35	0	28	0	0	0	0	0	0	0	0
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																

<b>Ring</b>	<b>1</b>	<b>2</b>	<b>3</b>	<b>4</b>
Split Sum	80s	80s	0s	0s

**Split Pattern # 9**

<b>Phase</b>	<b>1</b>	<b>2</b>	<b>3</b>	<b>4</b>	<b>5</b>	<b>6</b>	<b>7</b>	<b>8</b>	<b>9</b>	<b>10</b>	<b>11</b>	<b>12</b>	<b>13</b>	<b>14</b>	<b>15</b>	<b>16</b>
Description																
Split (seconds)	15	30	0	25	15	30	0	25	0	0	0	0	0	0	0	0
Coord Phase		X				X										
Vehicle Recall																
Pedestrian Recall																
Recall to Max. Time																
Omit Phase																

<b>Ring</b>	<b>1</b>	<b>2</b>	<b>3</b>	<b>4</b>
Split Sum	70s	70s	0s	0s

Ramsey County, MN



MOVING TRAFFIC FORWARD

Co Rd D @ Southlawn Dr - 672 - Econolite Type - ASC/3

**Time Base Action Plan  
Action Plan (MM) 5-2**

**Action Plan - 1**

Pattern	Free	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	0	Det Log	None
Flash	No	Red Rest	No
Veh Det Diag Plan	0	Ped Det Diag Plan	0
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																

Spec Func (1-8)								
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Aux Func (1-3)			
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	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 2**

Pattern	2	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 3**

Pattern	Free	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall		X				X										
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 4**

Pattern	4	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 5**

Pattern	5	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 6**

Pattern	6	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 7**

Pattern	Free	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall		X				X										
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 8**

Pattern	8	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 9**

Pattern	9	Override Sys	No
Timing Plan	1	Sequence	1
Veh Detector Plan	1	Det Log	15
Flash	No	Red Rest	No
Veh Det Diag Plan	1	Ped Det Diag Plan	1
Dimming Enable	No	Pmt Veh Priority Ret	No
Pmt Ped Priority Ret	No	Pmt Queue Delay	No
Pmt Cond Delay	No		

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 99**

Pattern Flash Override Sys No  
 Timing Plan 0 Sequence 0  
 Veh Detector Plan 0 Det Log None  
 Flash Yes Red Rest No  
 Veh Det Diag Plan 0 Ped Det Diag Plan 0  
 Dimming Enable No Pmt Veh Priority Ret No  
 Pmt Ped Priority Ret No Pmt Queue Delay No  
 Pmt Cond Delay No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall																
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

**Action Plan - 100**

Pattern Free Override Sys No  
 Timing Plan 1 Sequence 1  
 Veh Detector Plan 1 Det Log 15  
 Flash No Red Rest No  
 Veh Det Diag Plan 1 Ped Det Diag Plan 1  
 Dimming Enable No Pmt Veh Priority Ret No  
 Pmt Ped Priority Ret No Pmt Queue Delay No  
 Pmt Cond Delay No

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Ped Recall																
Walk 2																
Veh Ext 2																
Veh Recall		X				X										
Max Recall																
Max 2																
Max 3																
CS Inhibit																
Omit																
Spec Func (1-8)																
Aux Func (1-3)																
	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	
LP 1-15	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.

	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15
LP 16-30	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 31-45	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 46-60	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 61-75	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 76-90	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.
LP 91-100	.	.	.	.	.	.	.	.	.	.	.	.	.	.	.



## Ramsey County, MN



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*MOVING TRAFFIC FORWARD*

Co Rd D @ Southlawn Dr - 672 - Econolite Type - ASC/3

**Time Base Day Plan/Schedule**  
**Day Plan (MM) 5-3****Day Plan #1**

Event	Action Plan	Start Time
1	100	00:00
2	6	10:30
3	6	15:00
4	100	17:30

**Day Plan #2**

Event	Action Plan	Start Time
1	100	00:00
2	4	11:00
3	100	15:45

**Day Plan #3**

Event	Action Plan	Start Time
1	100	00:00

**Schedule (MM) 5-4**

**Schedule Number - 1**

Day Plan No.: 1

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
	X	X	X	X	X	X	X	X	X	X	X	X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X	X	X	X	X	

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	X	X	X	X	X	X	X	X	X	X	X
	12	13	14	15	16	17	18	19	20	21	22
	X	X	X	X	X	X	X	X	X	X	X
	23	24	25	26	27	28	29	30	31		
	X	X	X	X	X	X	X	X	X		

**Schedule Number - 2**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
	X	X	X	X	X	X	X	X	X	X	X	X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
							X

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	X	X	X	X	X	X	X	X	X	X	X
	12	13	14	15	16	17	18	19	20	21	22
	X	X	X	X	X	X	X	X	X	X	X
	23	24	25	26	27	28	29	30	31		
	X	X	X	X	X	X	X	X	X		

**Schedule Number - 3**

Day Plan No.: 3

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
	X	X	X	X	X	X	X	X	X	X	X	X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
	X						

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	X	X	X	X	X	X	X	X	X	X	X
	12	13	14	15	16	17	18	19	20	21	22

	X	X	X	X	X	X	X	X	X	X	X	X
	<b>23</b>	<b>24</b>	<b>25</b>	<b>26</b>	<b>27</b>	<b>28</b>	<b>29</b>	<b>30</b>	<b>31</b>			
	X	X	X	X	X	X	X	X	X			

**Schedule Number - 5**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
	X											

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X	X	X	X	X	

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	X										
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		

**Schedule Number - 6**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
	X											

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X					

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
		X									
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		

**Schedule Number - 7**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
					X							

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X					

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		
			X	X	X	X	X	X	X		

**Schedule Number - 8**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
							X					

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
						X	

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
			X								
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		

**Schedule Number - 9**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
							X					

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
	X	X	X	X	X	X	X

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
				X							
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		

**Schedule Number - 10**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
							X					

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X					

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
					X						
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		

**Schedule Number - 11**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
									X			

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X					

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	X	X	X	X	X	X	X				
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		

**Schedule Number - 12**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
											X	

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
					X		

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
											X
	23	24	25	26	27	28	29	30	31		
	X	X	X	X	X	X					

**Schedule Number - 13**

Day Plan No.: 1

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
											X	

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
						X	

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		
	X	X	X	X	X	X	X				

**Schedule Number - 14**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
												X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
						X	

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		
		X									

**Schedule Number - 15**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
												X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
	X	X	X	X	X	X	X

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		
			X								

**Schedule Number - 16**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
												X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
		X					

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		
				X							

**Schedule Number - 17**

Day Plan No.: 2

Month	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
												X

Day (DOW)	SUN	MON	TUE	WED	THU	FRI	SAT
						X	

Day (DOM)	1	2	3	4	5	6	7	8	9	10	11
	12	13	14	15	16	17	18	19	20	21	22
	23	24	25	26	27	28	29	30	31		
									X		

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## Enclave - Maplewood

Vistro File: N:\...\220916\_R0037066.00\_Enclave -  
Maplewood.vistro

Scenario 3 AM Existing

Report File: N:\...\AM Existing.pdf

9/24/2022

**Intersection Analysis Summary**

<b>ID</b>	<b>Intersection Name</b>	<b>Control Type</b>	<b>Method</b>	<b>Worst Mvmt</b>	<b>V/C</b>	<b>Delay (s/veh)</b>	<b>LOS</b>
1	County Road D and Southlawn Drive	Signalized	HCM 7th Edition	NB Left	0.212	6.4	A
2	Beam Avenue and Southlawn Drive	Signalized	HCM 7th Edition	SB Left	0.132	7.0	A

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report**  
**Intersection 1: County Road D and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	6.4
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.212

**Intersection Setup**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵↑			↑↵			↵↑↑			↵↑↑		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	0	0	1	1	0	1	1	0	0
Entry Pocket Length [ft]	200.00	100.00	100.00	100.00	100.00	50.00	150.00	100.00	125.00	325.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Curb Present	No			No			No			No		
Crosswalk	No			No			No			No		

**Volumes**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D		
Base Volume Input [veh/h]	5	0	21	1	0	2	3	216	11	53	190	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00											
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	5	0	21	1	0	2	3	216	11	53	190	1
Peak Hour Factor	0.6250	1.0000	0.5830	0.2500	1.0000	0.5000	0.7500	0.9000	0.6880	0.7790	0.8800	0.2500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	0	9	1	0	1	1	60	4	17	54	1
Total Analysis Volume [veh/h]	8	0	36	4	0	4	4	240	16	68	216	4
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0			0		
v_di, Inbound Pedestrian Volume crossing m	0			0			0			0		
v_co, Outbound Pedestrian Volume crossing	0			0			0			0		
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0			0		
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	ProtPer	Permiss	Permiss	ProtPer	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	7	0	0	7	0	5	15	0	5	15	0
Maximum Green [s]	0	15	0	0	15	0	15	60	0	15	60	0
Amber [s]	0.0	3.0	0.0	0.0	3.2	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	2.2	0.0	0.0	2.0	0.0	2.0	1.4	0.0	2.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	2.7	0.0	0.0	2.0	0.0	2.0	3.5	0.0	2.0	3.5	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	20	0	0	23	0	0	20	0	0	11	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No			No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	3.2	0.0	0.0	3.2	0.0	3.0	3.4	0.0	3.0	3.0	0.0
Minimum Recall		No			No		No	No		No	No	
Maximum Recall		No			No		No	No		No	No	
Pedestrian Recall		No			No		No	No		No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	C	R	L	C	R	L	C	C
C, Cycle Length [s]	34	34	34	34	34	34	34	34	34	34
L, Total Lost Time per Cycle [s]	5.20	5.20	5.20	5.20	5.40	5.40	5.40	5.00	5.00	5.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	3.20	3.20	3.20	3.20	0.00	3.40	3.40	0.00	3.00	3.00
g_i, Effective Green Time [s]	2	2	2	2	21	14	14	22	16	16
g / C, Green / Cycle	0.07	0.07	0.07	0.07	0.62	0.40	0.40	0.63	0.48	0.48
(v / s)_i Volume / Saturation Flow Rate	0.01	0.02	0.12	0.00	0.00	0.07	0.01	0.04	0.06	0.06
s, saturation flow rate [veh/h]	1412	1589	32	1589	1254	3560	1589	1781	1870	1858
c, Capacity [veh/h]	216	114	212	114	1061	1433	640	1374	896	891
d1, Uniform Delay [s]	17.11	15.13	17.15	14.82	2.50	6.57	6.19	2.45	4.94	4.95
k, delay calibration	0.09	0.09	0.04	0.04	0.04	0.13	0.13	0.13	0.13	0.13
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.06	1.33	0.01	0.05	0.00	0.07	0.02	0.02	0.07	0.07
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.04	0.32	0.02	0.04	0.00	0.17	0.03	0.05	0.12	0.12
d, Delay for Lane Group [s/veh]	17.17	16.46	17.16	14.87	2.50	6.64	6.21	2.47	5.02	5.02
Lane Group LOS	B	B	B	B	A	A	A	A	A	A
Critical Lane Group	No	No	Yes	No	No	Yes	No	Yes	No	No
50th-Percentile Queue Length [veh/ln]	0.06	0.26	0.03	0.03	0.00	0.37	0.05	0.05	0.26	0.26
50th-Percentile Queue Length [ft/ln]	1.41	6.53	0.68	0.63	0.08	9.32	1.21	1.27	6.51	6.50
95th-Percentile Queue Length [veh/ln]	0.10	0.47	0.05	0.05	0.01	0.67	0.09	0.09	0.47	0.47
95th-Percentile Queue Length [ft/ln]	2.54	11.75	1.23	1.14	0.14	16.78	2.18	2.29	11.72	11.69

**Movement, Approach, & Intersection Results**

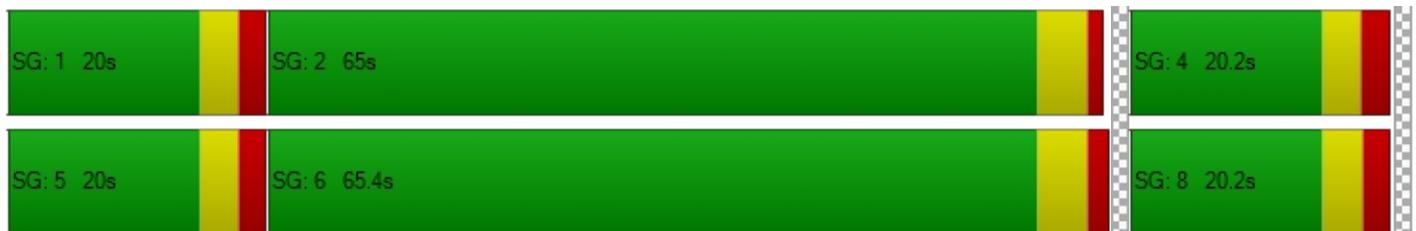
d_M, Delay for Movement [s/veh]	17.17	16.46	16.46	17.16	17.16	14.87	2.50	6.64	6.21	2.47	5.02	5.02
Movement LOS	B	B	B	B	B	B	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	16.59			16.02			6.55			4.42		
Approach LOS	B			B			A			A		
d_I, Intersection Delay [s/veh]	6.39											
Intersection LOS	A											
Intersection V/C	0.212											

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0		
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00		
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000		
Crosswalk LOS	F			F			F			F		
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000		
c_b, Capacity of the bicycle lane [bicycles/h]	877			877			3508			3508		
d_b, Bicycle Delay [s]	5.39			5.39			9.73			9.73		
I_b,int, Bicycle LOS Score for Intersection	1.632			1.573			1.774			1.797		
Bicycle LOS	A			A			A			A		

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 2: Beam Avenue and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	7.0
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.132

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive			Beam Avenue				Beam Avenue			
Approach	Northbound			Southbound			Eastbound				Westbound			
Lane Configuration	⇐⇐⇐			⇐⇐⇐			⇐⇐⇐⇐⇐				⇐⇐⇐⇐⇐			
Turning Movement	Left	Thru	Right	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	1	1	0	0	1	1	0	0	1
Entry Pocket Length [ft]	125.00	100.00	100.00	225.00	100.00	275.00	325.0	100.0	100.0	225.0	325.0	100.0	100.0	125.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00				30.00			
Grade [%]	0.00			0.00			0.00				0.00			
Curb Present	No			No			No				No			
Crosswalk	No			No			No				No			

**Volumes**

Name	Southlawn Drive			Southlawn Drive			Beam Avenue				Beam Avenue			
Base Volume Input [veh/h]	18	4	18	24	14	17	4	53	247	20	7	15	213	36
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00													
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	18	4	18	24	14	17	4	53	247	20	7	15	213	36
Peak Hour Factor	0.6430	0.2500	0.9000	0.7500	0.5830	0.7080	0.500	0.697	0.813	0.625	0.583	0.536	0.750	0.692
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	7	4	5	8	6	6	2	19	76	8	3	7	71	13
Total Analysis Volume [veh/h]	28	16	20	32	24	24	8	76	304	32	12	28	284	52
Presence of On-Street Parking	No		No	No		No	No			No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0				0			
v_di, Inbound Pedestrian Volume crossing m	0			0			0				0			
v_co, Outbound Pedestrian Volume crossing	0			0			0				0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0				0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0				0			
Bicycle Volume [bicycles/h]	0			0			0				0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permi	ProtP	Permi	Permi	Permi	ProtP	Permi	Permi
Signal Group	0	8	0	0	4	0	0	5	2	0	0	1	6	0
Auxiliary Signal Groups														
Lead / Lag	-	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	10	0	0	10	0	0	5	10	0	0	5	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	30	0	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	0	5	0	0	5	0	0	0	5	0	0	0	5	0
Pedestrian Clearance [s]	0	10	0	0	10	0	0	0	10	0	0	0	10	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No	No			No	No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
Minimum Recall		No			No			No	No			No	No	
Maximum Recall		No			No			No	No			No	No	
Pedestrian Recall		No			No			No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	L	C	R	L	C	R	L	C	R
C, Cycle Length [s]	29	29	29	29	29	29	29	29	29	29	29
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00
g_i, Effective Green Time [s]	5	5	5	5	5	16	10	10	16	9	9
g / C, Green / Cycle	0.17	0.17	0.17	0.17	0.17	0.55	0.36	0.36	0.55	0.33	0.33
(v / s)_i Volume / Saturation Flow Rate	0.02	0.02	0.02	0.01	0.02	0.07	0.09	0.02	0.03	0.08	0.03
s, saturation flow rate [veh/h]	1357	1703	1372	1870	1589	1254	3560	1589	1203	3560	1589
c, Capacity [veh/h]	383	289	374	317	269	1008	1296	579	980	1161	518
d1, Uniform Delay [s]	11.97	10.16	12.20	10.07	10.09	3.07	6.37	5.95	2.99	7.11	6.77
k, delay calibration	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.08	0.19	0.10	0.10	0.14	0.04	0.09	0.04	0.02	0.11	0.08
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.07	0.12	0.09	0.08	0.09	0.08	0.23	0.06	0.04	0.24	0.10
d, Delay for Lane Group [s/veh]	12.05	10.35	12.29	10.17	10.24	3.11	6.46	5.99	3.01	7.22	6.85
Lane Group LOS	B	B	B	B	B	A	A	A	A	A	A
Critical Lane Group	No	No	Yes	No	No	Yes	No	No	No	Yes	No
50th-Percentile Queue Length [veh/ln]	0.13	0.15	0.15	0.10	0.10	0.07	0.38	0.08	0.03	0.40	0.15
50th-Percentile Queue Length [ft/ln]	3.30	3.83	3.83	2.50	2.54	1.73	9.45	1.93	0.80	9.99	3.63
95th-Percentile Queue Length [veh/ln]	0.24	0.28	0.28	0.18	0.18	0.12	0.68	0.14	0.06	0.72	0.26
95th-Percentile Queue Length [ft/ln]	5.93	6.89	6.90	4.49	4.58	3.11	17.01	3.48	1.45	17.99	6.53

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	12.05	10.35	10.35	12.29	10.17	10.24	3.11	3.11	6.46	5.99	3.01	3.01	7.22	6.85
Movement LOS	B	B	B	B	B	B	A	A	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	11.09			11.04			5.76			6.72				
Approach LOS	B			B			A			A				
d_I, Intersection Delay [s/veh]	6.96													
Intersection LOS	A													
Intersection V/C	0.132													

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000			
Crosswalk LOS	F			F			F			F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000			
c_b, Capacity of the bicycle lane [bicycles/h]	2093			2093			2093			2093			
d_b, Bicycle Delay [s]	0.03			0.03			0.03			0.03			
I_b,int, Bicycle LOS Score for Intersection	1.665			1.692			1.843			1.847			
Bicycle LOS	A			A			A			A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Enclave - Maplewood

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Scenario 3 AM Existing

Report File: N:\...\AM Existing.pdf

9/24/2022

**Turning Movement Volume: Detail**

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
1	County Road D and Southlawn Drive	Final Base	5	0	21	1	0	2	3	216	11	53	190	1	503
		Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>5</b>	<b>0</b>	<b>21</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>3</b>	<b>216</b>	<b>11</b>	<b>53</b>	<b>190</b>	<b>1</b>	<b>503</b>

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound				
			Left	Thru	Right	Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right
2	Beam Avenue and Southlawn Drive	Final Base	18	4	18	24	14	17	4	53	247	20	7	15	213	36
		Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>18</b>	<b>4</b>	<b>18</b>	<b>24</b>	<b>14</b>	<b>17</b>	<b>4</b>	<b>53</b>	<b>247</b>	<b>20</b>	<b>7</b>	<b>15</b>	<b>213</b>	<b>36</b>

Total Volume
690
-
0
0
0
<b>690</b>

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## Enclave - Maplewood

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Maplewood.vistro

Scenario 4 PM Existing

Report File: N:\...\PM Existing.pdf

9/24/2022

**Intersection Analysis Summary**

<b>ID</b>	<b>Intersection Name</b>	<b>Control Type</b>	<b>Method</b>	<b>Worst Mvmt</b>	<b>V/C</b>	<b>Delay (s/veh)</b>	<b>LOS</b>
1	County Road D and Southlawn Drive	Signalized	HCM 7th Edition	NB Left	0.232	11.3	B
2	Beam Avenue and Southlawn Drive	Signalized	HCM 7th Edition	SB Left	0.272	9.6	A

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report**  
**Intersection 1: County Road D and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	11.3
Analysis Method:	HCM 7th Edition	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.232

**Intersection Setup**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D			
Approach	Northbound			Southbound			Eastbound			Westbound			
Lane Configuration	↵↑			↑↵			↵↵↵			↵↑			
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	0	0	1	1	0	1	1	0	0	0
Entry Pocket Length [ft]	200.00	100.00	100.00	100.00	100.00	50.00	150.00	100.00	125.00	325.0	100.0	100.0	100.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00			
Grade [%]	0.00			0.00			0.00			0.00			
Curb Present	No			No			No			No			
Crosswalk	No			No			No			No			

**Volumes**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D			
Base Volume Input [veh/h]	23	6	89	29	7	28	36	430	35	1	53	244	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00												
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	23	6	89	29	7	28	36	430	35	1	53	244	28
Peak Hour Factor	0.5750	0.5000	0.8560	0.9060	0.4380	0.8750	0.8180	0.8810	0.6730	0.250	0.722	0.924	0.778
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	10	3	26	8	4	8	11	122	13	1	18	66	9
Total Analysis Volume [veh/h]	40	12	104	32	16	32	44	488	52	4	73	264	36
Presence of On-Street Parking	No		No	No		No	No		No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0			0			
v_di, Inbound Pedestrian Volume crossing m	0			0			0			0			
v_co, Outbound Pedestrian Volume crossing	0			0			0			0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0			0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0			0			
Bicycle Volume [bicycles/h]	0			0			0			0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Semi-actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	ProtPer	Permiss	Permiss	Permi	ProtP	Permi	Permi
Signal Group	0	4	0	0	8	0	1	6	0	0	5	2	0
Auxiliary Signal Groups													
Lead / Lag	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	7	0	0	7	0	5	15	0	0	5	15	0
Maximum Green [s]	0	15	0	0	15	0	15	60	0	0	15	60	0
Amber [s]	0.0	3.0	0.0	0.0	3.2	0.0	3.0	4.0	0.0	0.0	3.0	4.0	0.0
All red [s]	0.0	2.2	0.0	0.0	2.0	0.0	2.0	1.4	0.0	0.0	2.0	1.0	0.0
Split [s]	0	18	0	0	18	0	12	29	0	0	13	30	0
Vehicle Extension [s]	0.0	2.7	0.0	0.0	2.0	0.0	2.0	3.5	0.0	0.0	2.0	3.5	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	0	7	0
Pedestrian Clearance [s]	0	20	0	0	23	0	0	20	0	0	0	11	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No				No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	3.2	0.0	0.0	3.2	0.0	3.0	3.4	0.0	0.0	3.0	3.0	0.0
Minimum Recall		No			No		No	No			No	No	
Maximum Recall		No			No		No	No			No	No	
Pedestrian Recall		No			No		No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	C	R	L	C	R	L	C	C
C, Cycle Length [s]	60	60	60	60	60	60	60	60	60	60
L, Total Lost Time per Cycle [s]	5.20	5.20	5.20	5.20	5.40	5.40	5.40	5.00	5.00	5.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	3.20	3.20	3.20	3.20	0.00	3.40	3.40	0.00	3.00	3.00
g_i, Effective Green Time [s]	12	12	12	12	37	29	29	38	30	30
g / C, Green / Cycle	0.20	0.20	0.20	0.20	0.62	0.48	0.48	0.63	0.50	0.50
(v / s)_i Volume / Saturation Flow Rate	0.03	0.07	0.06	0.02	0.04	0.14	0.03	0.04	0.08	0.08
s, saturation flow rate [veh/h]	1357	1615	851	1589	1184	3560	1589	1781	1870	1793
c, Capacity [veh/h]	181	322	270	317	877	1710	763	1123	942	903
d1, Uniform Delay [s]	28.18	20.71	20.86	19.62	4.40	9.39	8.38	4.28	8.04	8.05
k, delay calibration	0.09	0.09	0.04	0.04	0.04	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.52	0.58	0.12	0.05	0.01	0.42	0.17	0.12	0.37	0.39
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.22	0.36	0.18	0.10	0.05	0.29	0.07	0.07	0.16	0.16
d, Delay for Lane Group [s/veh]	28.70	21.29	20.98	19.67	4.41	9.81	8.55	4.40	8.41	8.44
Lane Group LOS	C	C	C	B	A	A	A	A	A	A
Critical Lane Group	No	Yes	No	No	No	Yes	No	Yes	No	No
50th-Percentile Queue Length [veh/ln]	0.56	1.36	0.52	0.35	0.16	1.74	0.35	0.30	0.99	0.97
50th-Percentile Queue Length [ft/ln]	14.07	33.94	13.09	8.64	3.88	43.41	8.69	7.53	24.71	24.20
95th-Percentile Queue Length [veh/ln]	1.01	2.44	0.94	0.62	0.28	3.13	0.63	0.54	1.78	1.74
95th-Percentile Queue Length [ft/ln]	25.32	61.10	23.55	15.56	6.99	78.13	15.65	13.55	44.48	43.57

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	28.70	21.29	21.29	20.98	20.98	19.67	4.41	9.81	8.55	4.40	4.40	8.42	8.44
Movement LOS	C	C	C	C	C	B	A	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	23.19			20.45			9.29			7.60			
Approach LOS	C			C			A			A			
d_I, Intersection Delay [s/veh]	11.32												
Intersection LOS	B												
Intersection V/C	0.232												

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000			
Crosswalk LOS	F			F			F			F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000			
c_b, Capacity of the bicycle lane [bicycles/h]	427			427			787			833			
d_b, Bicycle Delay [s]	18.57			18.57			11.04			10.21			
I_b,int, Bicycle LOS Score for Intersection	1.817			1.692			2.041			1.867			
Bicycle LOS	A			A			B			A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 2: Beam Avenue and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	9.6
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.272

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive				Beam Avenue				Beam Avenue			
Approach	Northbound			Southbound				Eastbound				Westbound			
Lane Configuration	T T			T L R				T L R				T L R			
Turning Movement	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	1	0	0	1	1	0	0	1
Entry Pocket Length [ft]	125.00	100.00	100.00	225.0	100.0	100.0	275.0	325.0	100.0	100.0	225.0	325.0	100.0	100.0	125.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00				30.00				30.00			
Grade [%]	0.00			0.00				0.00				0.00			
Curb Present	No			No				No				No			
Crosswalk	No			No				No				No			

**Volumes**

Name	Southlawn Drive			Southlawn Drive				Beam Avenue				Beam Avenue			
Base Volume Input [veh/h]	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00														
Growth Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
Peak Hour Factor	0.7920	0.4170	0.8750	0.250	0.884	0.607	0.692	0.750	0.859	0.934	0.712	0.750	0.750	0.879	0.679
Other Adjustment Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	12	6	10	1	28	7	39	2	46	137	13	1	12	91	28
Total Analysis Volume [veh/h]	48	24	40	4	112	28	156	8	184	548	52	4	48	364	112
Presence of On-Street Parking	No		No	No			No	No			No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0				0				0			
v_di, Inbound Pedestrian Volume crossing m	0			0				0				0			
v_co, Outbound Pedestrian Volume crossing	0			0				0				0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0				0				0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0				0				0			
Bicycle Volume [bicycles/h]	0			0				0				0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permi	Permi	Permi	Permi	Permi	ProtP	Permi	Permi	Permi	ProtP	Permi	Permi
Signal Group	0	8	0	0	0	4	0	0	5	2	0	0	1	6	0
Auxiliary Signal Groups															
Lead / Lag	-	-	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	10	0	0	0	10	0	0	5	10	0	0	5	10	0
Maximum Green [s]	0	30	0	0	0	30	0	0	30	30	0	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	0	5	0	0	0	5	0	0	0	5	0	0	0	5	0
Pedestrian Clearance [s]	0	10	0	0	0	10	0	0	0	10	0	0	0	10	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No				No				No				No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
Minimum Recall		No				No			No	No			No	No	
Maximum Recall		No				No			No	No			No	No	
Pedestrian Recall		No				No			No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	L	C	R	L	C	R	L	C	R
C, Cycle Length [s]	36	36	36	36	36	36	36	36	36	36	36
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00
g_i, Effective Green Time [s]	10	10	10	10	10	18	12	12	18	10	10
g / C, Green / Cycle	0.27	0.27	0.27	0.27	0.27	0.51	0.34	0.34	0.51	0.28	0.28
(v / s)_i Volume / Saturation Flow Rate	0.04	0.04	0.09	0.01	0.10	0.16	0.15	0.03	0.05	0.10	0.07
s, saturation flow rate [veh/h]	1200	1684	1338	1870	1589	1218	3560	1589	1036	3560	1589
c, Capacity [veh/h]	448	449	452	499	424	848	1209	540	730	988	441
d1, Uniform Delay [s]	11.79	10.04	12.97	9.80	10.71	4.97	9.26	8.10	4.71	10.44	10.09
k, delay calibration	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.10	0.14	0.30	0.05	0.53	0.13	0.27	0.08	0.04	0.23	0.30
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.11	0.14	0.26	0.06	0.37	0.23	0.45	0.10	0.07	0.37	0.25
d, Delay for Lane Group [s/veh]	11.90	10.18	13.27	9.85	11.24	5.10	9.52	8.17	4.75	10.67	10.39
Lane Group LOS	B	B	B	A	B	A	A	A	A	B	B
Critical Lane Group	No	No	No	No	Yes	Yes	No	No	No	Yes	No
50th-Percentile Queue Length [veh/ln]	0.26	0.31	0.70	0.13	0.83	0.46	1.24	0.21	0.11	0.90	0.55
50th-Percentile Queue Length [ft/ln]	6.60	7.78	17.44	3.29	20.64	11.49	30.89	5.23	2.87	22.58	13.87
95th-Percentile Queue Length [veh/ln]	0.47	0.56	1.26	0.24	1.49	0.83	2.22	0.38	0.21	1.63	1.00
95th-Percentile Queue Length [ft/ln]	11.87	14.01	31.38	5.93	37.15	20.69	55.61	9.42	5.17	40.64	24.96

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	11.90	10.18	10.18	13.27	13.27	9.85	11.24	5.10	5.10	9.52	8.17	4.75	4.75	10.67	10.39
Movement LOS	B	B	B	B	B	A	B	A	A	A	A	A	A	B	B
d_A, Approach Delay [s/veh]	10.92			11.89			8.36			10.03					
Approach LOS	B			B			A			B					
d_I, Intersection Delay [s/veh]	9.65														
Intersection LOS	A														
Intersection V/C	0.272														

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0		
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00		
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000		
Crosswalk LOS	F			F			F			F		
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000		
c_b, Capacity of the bicycle lane [bicycles/h]	1676			1676			1676			1676		
d_b, Bicycle Delay [s]	0.47			0.47			0.47			0.47		
I_b,int, Bicycle LOS Score for Intersection	1.744			1.870			2.061			1.956		
Bicycle LOS	A			A			B			A		

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Enclave - Maplewood

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Scenario 4 PM Existing

Report File: N:\...\PM Existing.pdf

9/24/2022

**Turning Movement Volume: Detail**

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume		
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-T	Left	Thru		Right	
1	County Road D and Southlawn Drive	Final Base	23	6	89	29	7	28	36	430	35	1	53	244	28	1009	
		Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>23</b>	<b>6</b>	<b>89</b>	<b>29</b>	<b>7</b>	<b>28</b>	<b>36</b>	<b>430</b>	<b>35</b>	<b>1</b>	<b>53</b>	<b>244</b>	<b>28</b>	<b>1009</b>	

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound					
			Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right
2	Beam Avenue and Southlawn Drive	Final Base	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
		Growth Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>38</b>	<b>10</b>	<b>35</b>	<b>1</b>	<b>99</b>	<b>17</b>	<b>108</b>	<b>6</b>	<b>158</b>	<b>512</b>	<b>37</b>	<b>3</b>	<b>36</b>	<b>320</b>	<b>76</b>

Total Volume
1456
-
0
0
0
<b>1456</b>

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## Enclave - Maplewood

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Maplewood.vistro

Scenario 9 AM 2024 Background

Report File: N:\...\AM Future Background.pdf

9/24/2022

**Intersection Analysis Summary**

<b>ID</b>	<b>Intersection Name</b>	<b>Control Type</b>	<b>Method</b>	<b>Worst Mvmt</b>	<b>V/C</b>	<b>Delay (s/veh)</b>	<b>LOS</b>
1	County Road D and Southlawn Drive	Signalized	HCM 7th Edition	NB Left	0.213	6.4	A
2	Beam Avenue and Southlawn Drive	Signalized	HCM 7th Edition	SB Left	0.133	7.0	A

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report**  
**Intersection 1: County Road D and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	6.4
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.213

**Intersection Setup**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵↑			↑↵			↵↑↑			↵↑↑		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	0	0	1	1	0	1	1	0	0
Entry Pocket Length [ft]	200.00	100.00	100.00	100.00	100.00	50.00	150.00	100.00	125.00	325.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Curb Present	No			No			No			No		
Crosswalk	No			No			No			No		

**Volumes**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D		
Base Volume Input [veh/h]	5	0	21	1	0	2	3	216	11	53	190	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00											
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	5	0	21	1	0	2	3	218	11	54	192	1
Peak Hour Factor	0.6250	1.0000	0.5830	0.2500	1.0000	0.5000	0.7500	0.9000	0.6880	0.7790	0.8800	0.2500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	2	0	9	1	0	1	1	61	4	17	55	1
Total Analysis Volume [veh/h]	8	0	36	4	0	4	4	242	16	69	218	4
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0			0		
v_di, Inbound Pedestrian Volume crossing m	0			0			0			0		
v_co, Outbound Pedestrian Volume crossing	0			0			0			0		
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0			0		
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	ProtPer	Permiss	Permiss	ProtPer	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	7	0	0	7	0	5	15	0	5	15	0
Maximum Green [s]	0	15	0	0	15	0	15	60	0	15	60	0
Amber [s]	0.0	3.0	0.0	0.0	3.2	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	2.2	0.0	0.0	2.0	0.0	2.0	1.4	0.0	2.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	2.7	0.0	0.0	2.0	0.0	2.0	3.5	0.0	2.0	3.5	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	20	0	0	23	0	0	20	0	0	11	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No			No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	3.2	0.0	0.0	3.2	0.0	3.0	3.4	0.0	3.0	3.0	0.0
Minimum Recall		No			No		No	No		No	No	
Maximum Recall		No			No		No	No		No	No	
Pedestrian Recall		No			No		No	No		No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	C	R	L	C	R	L	C	C
C, Cycle Length [s]	34	34	34	34	34	34	34	34	34	34
L, Total Lost Time per Cycle [s]	5.20	5.20	5.20	5.20	5.40	5.40	5.40	5.00	5.00	5.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	3.20	3.20	3.20	3.20	0.00	3.40	3.40	0.00	3.00	3.00
g_i, Effective Green Time [s]	2	2	2	2	21	14	14	22	16	16
g / C, Green / Cycle	0.07	0.07	0.07	0.07	0.62	0.40	0.40	0.63	0.48	0.48
(v / s)_i Volume / Saturation Flow Rate	0.01	0.02	0.12	0.00	0.00	0.07	0.01	0.04	0.06	0.06
s, saturation flow rate [veh/h]	1412	1589	32	1589	1252	3560	1589	1781	1870	1858
c, Capacity [veh/h]	216	114	212	114	1060	1433	640	1374	898	892
d1, Uniform Delay [s]	17.14	15.16	17.18	14.85	2.50	6.58	6.20	2.45	4.94	4.94
k, delay calibration	0.09	0.09	0.04	0.04	0.04	0.13	0.13	0.13	0.13	0.13
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.06	1.33	0.01	0.05	0.00	0.07	0.02	0.02	0.07	0.07
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.04	0.32	0.02	0.04	0.00	0.17	0.03	0.05	0.12	0.12
d, Delay for Lane Group [s/veh]	17.20	16.49	17.19	14.90	2.50	6.65	6.22	2.47	5.01	5.02
Lane Group LOS	B	B	B	B	A	A	A	A	A	A
Critical Lane Group	No	No	Yes	No	No	Yes	No	Yes	No	No
50th-Percentile Queue Length [veh/ln]	0.06	0.26	0.03	0.03	0.00	0.38	0.05	0.05	0.26	0.26
50th-Percentile Queue Length [ft/ln]	1.42	6.54	0.68	0.63	0.08	9.43	1.22	1.30	6.58	6.56
95th-Percentile Queue Length [veh/ln]	0.10	0.47	0.05	0.05	0.01	0.68	0.09	0.09	0.47	0.47
95th-Percentile Queue Length [ft/ln]	2.55	11.78	1.23	1.14	0.14	16.98	2.19	2.33	11.84	11.81

**Movement, Approach, & Intersection Results**

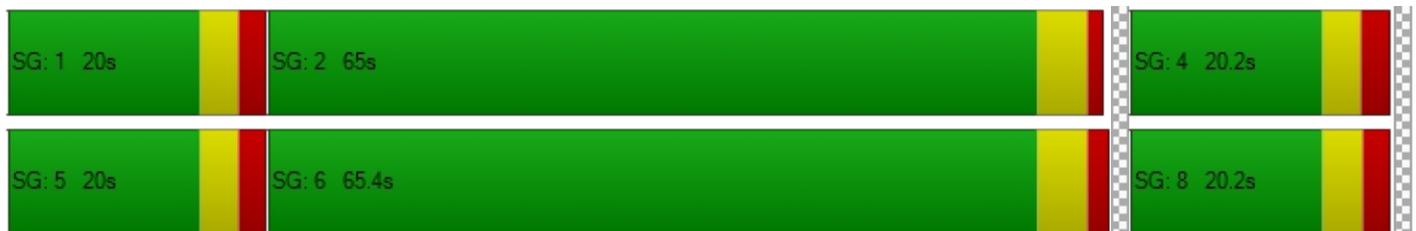
d_M, Delay for Movement [s/veh]	17.20	16.49	16.49	17.19	17.19	14.90	2.50	6.65	6.22	2.47	5.02	5.02
Movement LOS	B	B	B	B	B	B	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	16.62			16.04			6.56			4.41		
Approach LOS	B			B			A			A		
d_I, Intersection Delay [s/veh]	6.38											
Intersection LOS	A											
Intersection V/C	0.213											

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0		
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00		
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000		
Crosswalk LOS	F			F			F			F		
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000		
c_b, Capacity of the bicycle lane [bicycles/h]	875			875			3502			3502		
d_b, Bicycle Delay [s]	5.42			5.42			9.66			9.66		
I_b,int, Bicycle LOS Score for Intersection	1.632			1.573			1.776			1.800		
Bicycle LOS	A			A			A			A		

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 2: Beam Avenue and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	7.0
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.133

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive			Beam Avenue				Beam Avenue			
Approach	Northbound			Southbound			Eastbound				Westbound			
Lane Configuration	⇐ ⇈			⇈ ⇐			⇈ ⇐ ⇈				⇈ ⇐ ⇈			
Turning Movement	Left	Thru	Right	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	1	1	0	0	1	1	0	0	1
Entry Pocket Length [ft]	125.00	100.00	100.00	225.00	100.00	275.00	325.0	100.0	100.0	225.0	325.0	100.0	100.0	125.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00				30.00			
Grade [%]	0.00			0.00			0.00				0.00			
Curb Present	No			No			No				No			
Crosswalk	No			No			No				No			

**Volumes**

Name	Southlawn Drive			Southlawn Drive			Beam Avenue				Beam Avenue			
Base Volume Input [veh/h]	18	4	18	24	14	17	4	53	247	20	7	15	213	36
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00													
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	18	4	18	24	14	17	4	54	249	20	7	15	215	36
Peak Hour Factor	0.6430	0.2500	0.9000	0.7500	0.5830	0.7080	0.500	0.697	0.813	0.625	0.583	0.536	0.750	0.692
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	7	4	5	8	6	6	2	19	77	8	3	7	72	13
Total Analysis Volume [veh/h]	28	16	20	32	24	24	8	77	306	32	12	28	287	52
Presence of On-Street Parking	No		No	No		No	No			No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0				0			
v_di, Inbound Pedestrian Volume crossing m	0			0			0				0			
v_co, Outbound Pedestrian Volume crossing	0			0			0				0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0				0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0				0			
Bicycle Volume [bicycles/h]	0			0			0				0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permi	ProtP	Permi	Permi	Permi	ProtP	Permi	Permi
Signal Group	0	8	0	0	4	0	0	5	2	0	0	1	6	0
Auxiliary Signal Groups														
Lead / Lag	-	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	10	0	0	10	0	0	5	10	0	0	5	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	30	0	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	0	5	0	0	5	0	0	0	5	0	0	0	5	0
Pedestrian Clearance [s]	0	10	0	0	10	0	0	0	10	0	0	0	10	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No	No			No	No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
Minimum Recall		No			No			No	No			No	No	
Maximum Recall		No			No			No	No			No	No	
Pedestrian Recall		No			No			No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	L	C	R	L	C	R	L	C	R
C, Cycle Length [s]	29	29	29	29	29	29	29	29	29	29	29
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00
g_i, Effective Green Time [s]	5	5	5	5	5	16	10	10	16	9	9
g / C, Green / Cycle	0.17	0.17	0.17	0.17	0.17	0.55	0.37	0.37	0.55	0.33	0.33
(v / s)_i Volume / Saturation Flow Rate	0.02	0.02	0.02	0.01	0.02	0.07	0.09	0.02	0.03	0.08	0.03
s, saturation flow rate [veh/h]	1357	1703	1372	1870	1589	1252	3560	1589	1201	3560	1589
c, Capacity [veh/h]	383	288	374	317	269	1007	1299	580	978	1161	518
d1, Uniform Delay [s]	11.99	10.18	12.22	10.09	10.11	3.07	6.37	5.94	2.99	7.13	6.78
k, delay calibration	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.08	0.19	0.10	0.10	0.14	0.04	0.09	0.04	0.02	0.11	0.08
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.07	0.12	0.09	0.08	0.09	0.08	0.24	0.06	0.04	0.25	0.10
d, Delay for Lane Group [s/veh]	12.07	10.37	12.31	10.19	10.25	3.11	6.46	5.98	3.01	7.24	6.86
Lane Group LOS	B	B	B	B	B	A	A	A	A	A	A
Critical Lane Group	No	No	Yes	No	No	Yes	No	No	No	Yes	No
50th-Percentile Queue Length [veh/ln]	0.13	0.15	0.15	0.10	0.10	0.07	0.38	0.08	0.03	0.41	0.15
50th-Percentile Queue Length [ft/ln]	3.30	3.84	3.84	2.50	2.55	1.75	9.53	1.94	0.81	10.14	3.64
95th-Percentile Queue Length [veh/ln]	0.24	0.28	0.28	0.18	0.18	0.13	0.69	0.14	0.06	0.73	0.26
95th-Percentile Queue Length [ft/ln]	5.95	6.91	6.92	4.51	4.59	3.16	17.15	3.48	1.45	18.25	6.55

**Movement, Approach, & Intersection Results**

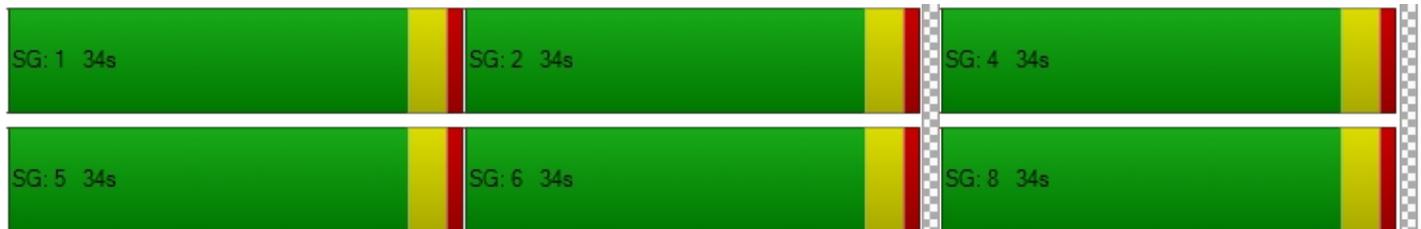
d_M, Delay for Movement [s/veh]	12.07	10.37	10.37	12.31	10.19	10.25	3.11	3.11	6.46	5.98	3.01	3.01	7.24	6.86
Movement LOS	B	B	B	B	B	B	A	A	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	11.11			11.06			5.75			6.74				
Approach LOS	B			B			A			A				
d_I, Intersection Delay [s/veh]	6.96													
Intersection LOS	A													
Intersection V/C	0.133													

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000			
Crosswalk LOS	F			F			F			F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000			
c_b, Capacity of the bicycle lane [bicycles/h]	2090			2090			2090			2090			
d_b, Bicycle Delay [s]	0.03			0.03			0.03			0.03			
I_b,int, Bicycle LOS Score for Intersection	1.665			1.692			1.845			1.849			
Bicycle LOS	A			A			A			A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Enclave - Maplewood

Vistro File: N:\...\220916\_R0037066.00\_Enclave - Maplewood.vistro

Scenario 9 AM 2024 Background

Report File: N:\...\AM Future Background.pdf

9/24/2022

**Turning Movement Volume: Detail**

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
1	County Road D and Southlawn Drive	Final Base	5	0	21	1	0	2	3	216	11	53	190	1	503
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>5</b>	<b>0</b>	<b>21</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>3</b>	<b>218</b>	<b>11</b>	<b>54</b>	<b>192</b>	<b>1</b>	<b>508</b>

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound				
			Left	Thru	Right	Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right
2	Beam Avenue and Southlawn Drive	Final Base	18	4	18	24	14	17	4	53	247	20	7	15	213	36
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>18</b>	<b>4</b>	<b>18</b>	<b>24</b>	<b>14</b>	<b>17</b>	<b>4</b>	<b>54</b>	<b>249</b>	<b>20</b>	<b>7</b>	<b>15</b>	<b>215</b>	<b>36</b>

Total Volume
690
-
0
0
0
<b>695</b>

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## Encalve - Maplewood

Vistro File: N:\...\220916\_R0037066.00\_Encalve -  
Maplewood.vistro

Scenario 10 PM 2024 Background

Report File: N:\...\PM Future Background.pdf

9/24/2022

**Intersection Analysis Summary**

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	County Road D and Southlawn Drive	Signalized	HCM 7th Edition	NB Left	0.235	11.3	B
2	Beam Avenue and Southlawn Drive	Signalized	HCM 7th Edition	SB Left	0.275	9.7	A

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report**  
**Intersection 1: County Road D and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	11.3
Analysis Method:	HCM 7th Edition	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.235

**Intersection Setup**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D			
Approach	Northbound			Southbound			Eastbound			Westbound			
Lane Configuration	↵↑			↑↵			↵↵↵			↵↑			
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	0	0	1	1	0	1	1	0	0	0
Entry Pocket Length [ft]	200.00	100.00	100.00	100.00	100.00	50.00	150.00	100.00	125.00	325.0	100.0	100.0	100.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00			
Grade [%]	0.00			0.00			0.00			0.00			
Curb Present	No			No			No			No			
Crosswalk	No			No			No			No			

**Volumes**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D			
Base Volume Input [veh/h]	23	6	89	29	7	28	36	430	35	1	53	244	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00												
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.010	1.010	1.010	1.010
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	23	6	90	29	7	28	36	434	35	1	54	246	28
Peak Hour Factor	0.5750	0.5000	0.8560	0.9060	0.4380	0.8750	0.8180	0.8810	0.6730	0.250	0.722	0.924	0.778
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	10	3	26	8	4	8	11	123	13	1	19	67	9
Total Analysis Volume [veh/h]	40	12	105	32	16	32	44	493	52	4	75	266	36
Presence of On-Street Parking	No		No	No		No	No		No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0			0			
v_di, Inbound Pedestrian Volume crossing m	0			0			0			0			
v_co, Outbound Pedestrian Volume crossing	0			0			0			0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0			0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0			0			
Bicycle Volume [bicycles/h]	0			0			0			0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Semi-actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	ProtPer	Permiss	Permiss	Permi	ProtP	Permi	Permi
Signal Group	0	4	0	0	8	0	1	6	0	0	5	2	0
Auxiliary Signal Groups													
Lead / Lag	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	7	0	0	7	0	5	15	0	0	5	15	0
Maximum Green [s]	0	15	0	0	15	0	15	60	0	0	15	60	0
Amber [s]	0.0	3.0	0.0	0.0	3.2	0.0	3.0	4.0	0.0	0.0	3.0	4.0	0.0
All red [s]	0.0	2.2	0.0	0.0	2.0	0.0	2.0	1.4	0.0	0.0	2.0	1.0	0.0
Split [s]	0	18	0	0	18	0	12	29	0	0	13	30	0
Vehicle Extension [s]	0.0	2.7	0.0	0.0	2.0	0.0	2.0	3.5	0.0	0.0	2.0	3.5	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	0	7	0
Pedestrian Clearance [s]	0	20	0	0	23	0	0	20	0	0	0	11	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No				No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	3.2	0.0	0.0	3.2	0.0	3.0	3.4	0.0	0.0	3.0	3.0	0.0
Minimum Recall		No			No		No	No			No	No	
Maximum Recall		No			No		No	No			No	No	
Pedestrian Recall		No			No		No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	C	R	L	C	R	L	C	C
C, Cycle Length [s]	60	60	60	60	60	60	60	60	60	60
L, Total Lost Time per Cycle [s]	5.20	5.20	5.20	5.20	5.40	5.40	5.40	5.00	5.00	5.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	3.20	3.20	3.20	3.20	0.00	3.40	3.40	0.00	3.00	3.00
g_i, Effective Green Time [s]	12	12	12	12	37	29	29	38	30	30
g / C, Green / Cycle	0.20	0.20	0.20	0.20	0.62	0.48	0.48	0.63	0.50	0.50
(v / s)_i Volume / Saturation Flow Rate	0.03	0.07	0.06	0.02	0.04	0.14	0.03	0.04	0.08	0.08
s, saturation flow rate [veh/h]	1357	1614	850	1589	1183	3560	1589	1781	1870	1794
c, Capacity [veh/h]	182	324	270	319	875	1704	761	1121	940	902
d1, Uniform Delay [s]	28.16	20.68	20.82	19.57	4.42	9.47	8.43	4.31	8.07	8.09
k, delay calibration	0.09	0.09	0.04	0.04	0.04	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.52	0.58	0.12	0.05	0.01	0.43	0.17	0.12	0.37	0.39
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.22	0.36	0.18	0.10	0.05	0.29	0.07	0.07	0.16	0.17
d, Delay for Lane Group [s/veh]	28.67	21.26	20.93	19.62	4.43	9.90	8.61	4.43	8.45	8.48
Lane Group LOS	C	C	C	B	A	A	A	A	A	A
Critical Lane Group	No	Yes	No	No	No	Yes	No	Yes	No	No
50th-Percentile Queue Length [veh/ln]	0.56	1.37	0.52	0.35	0.16	1.77	0.35	0.31	1.00	0.98
50th-Percentile Queue Length [ft/ln]	14.06	34.21	13.07	8.63	3.90	44.13	8.73	7.76	24.96	24.44
95th-Percentile Queue Length [veh/ln]	1.01	2.46	0.94	0.62	0.28	3.18	0.63	0.56	1.80	1.76
95th-Percentile Queue Length [ft/ln]	25.31	61.58	23.52	15.54	7.02	79.44	15.72	13.97	44.92	44.00

**Movement, Approach, & Intersection Results**

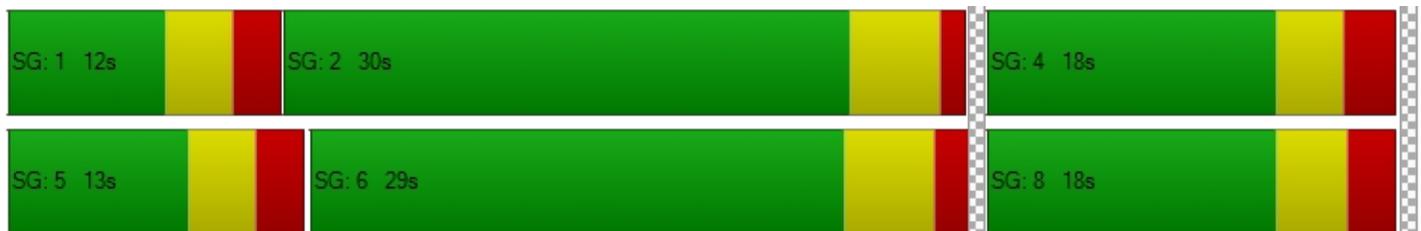
d_M, Delay for Movement [s/veh]	28.67	21.26	21.26	20.93	20.93	19.62	4.43	9.90	8.61	4.43	4.43	8.46	8.48
Movement LOS	C	C	C	C	C	B	A	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	23.15			20.41			9.38			7.63			
Approach LOS	C			C			A			A			
d_I, Intersection Delay [s/veh]	11.35												
Intersection LOS	B												
Intersection V/C	0.235												

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000			
Crosswalk LOS	F			F			F			F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000			
c_b, Capacity of the bicycle lane [bicycles/h]	427			427			787			833			
d_b, Bicycle Delay [s]	18.57			18.57			11.04			10.21			
I_b,int, Bicycle LOS Score for Intersection	1.819			1.692			2.046			1.871			
Bicycle LOS	A			A			B			A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 2: Beam Avenue and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	9.7
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.275

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive				Beam Avenue				Beam Avenue			
Approach	Northbound			Southbound				Eastbound				Westbound			
Lane Configuration	T T			T L R				T L R				T L R			
Turning Movement	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	1	0	0	1	1	0	0	1
Entry Pocket Length [ft]	125.00	100.00	100.00	225.0	100.0	100.0	275.0	325.0	100.0	100.0	225.0	325.0	100.0	100.0	125.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00				30.00				30.00			
Grade [%]	0.00			0.00				0.00				0.00			
Curb Present	No			No				No				No			
Crosswalk	No			No				No				No			

**Volumes**

Name	Southlawn Drive			Southlawn Drive				Beam Avenue				Beam Avenue			
Base Volume Input [veh/h]	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00														
Growth Factor	1.0100	1.0100	1.0100	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	38	10	35	1	100	17	109	6	160	517	37	3	36	323	77
Peak Hour Factor	0.7920	0.4170	0.8750	0.250	0.884	0.607	0.692	0.750	0.859	0.934	0.712	0.750	0.750	0.879	0.679
Other Adjustment Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	12	6	10	1	28	7	39	2	47	138	13	1	12	92	28
Total Analysis Volume [veh/h]	48	24	40	4	113	28	158	8	186	554	52	4	48	367	113
Presence of On-Street Parking	No		No	No			No	No			No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0				0				0			
v_di, Inbound Pedestrian Volume crossing m	0			0				0				0			
v_co, Outbound Pedestrian Volume crossing	0			0				0				0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0				0				0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0				0				0			
Bicycle Volume [bicycles/h]	0			0				0				0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permi	Permi	Permi	Permi	Permi	ProtP	Permi	Permi	Permi	ProtP	Permi	Permi
Signal Group	0	8	0	0	0	4	0	0	5	2	0	0	1	6	0
Auxiliary Signal Groups															
Lead / Lag	-	-	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	10	0	0	0	10	0	0	5	10	0	0	5	10	0
Maximum Green [s]	0	30	0	0	0	30	0	0	30	30	0	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	0	5	0	0	0	5	0	0	0	5	0	0	0	5	0
Pedestrian Clearance [s]	0	10	0	0	0	10	0	0	0	10	0	0	0	10	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No				No				No				No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
Minimum Recall		No				No			No	No			No	No	
Maximum Recall		No				No			No	No			No	No	
Pedestrian Recall		No				No			No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	L	C	R	L	C	R	L	C	R
C, Cycle Length [s]	36	36	36	36	36	36	36	36	36	36	36
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00
g_i, Effective Green Time [s]	10	10	10	10	10	18	12	12	18	10	10
g / C, Green / Cycle	0.27	0.27	0.27	0.27	0.27	0.51	0.34	0.34	0.51	0.28	0.28
(v / s)_i Volume / Saturation Flow Rate	0.04	0.04	0.09	0.01	0.10	0.16	0.16	0.03	0.05	0.10	0.07
s, saturation flow rate [veh/h]	1197	1684	1338	1870	1589	1216	3560	1589	1032	3560	1589
c, Capacity [veh/h]	447	450	452	499	424	846	1210	540	727	988	441
d1, Uniform Delay [s]	11.80	10.04	12.99	9.81	10.73	4.98	9.28	8.10	4.72	10.47	10.11
k, delay calibration	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.10	0.14	0.30	0.05	0.54	0.14	0.27	0.08	0.04	0.23	0.30
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.11	0.14	0.26	0.06	0.37	0.23	0.46	0.10	0.07	0.37	0.26
d, Delay for Lane Group [s/veh]	11.91	10.19	13.29	9.85	11.27	5.12	9.55	8.18	4.76	10.70	10.41
Lane Group LOS	B	B	B	A	B	A	A	A	A	B	B
Critical Lane Group	No	No	No	No	Yes	Yes	No	No	No	Yes	No
50th-Percentile Queue Length [veh/ln]	0.26	0.31	0.70	0.13	0.84	0.47	1.25	0.21	0.12	0.91	0.56
50th-Percentile Queue Length [ft/ln]	6.60	7.79	17.62	3.30	20.96	11.65	31.34	5.24	2.88	22.83	14.03
95th-Percentile Queue Length [veh/ln]	0.48	0.56	1.27	0.24	1.51	0.84	2.26	0.38	0.21	1.64	1.01
95th-Percentile Queue Length [ft/ln]	11.89	14.02	31.72	5.94	37.74	20.98	56.41	9.43	5.18	41.10	25.26

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	11.91	10.19	10.19	13.29	13.29	9.85	11.27	5.12	5.12	9.55	8.18	4.76	4.76	10.70	10.41
Movement LOS	B	B	B	B	B	A	B	A	A	A	A	A	A	B	B
d_A, Approach Delay [s/veh]	10.92			11.92			8.39			10.06					
Approach LOS	B			B			A			B					
d_I, Intersection Delay [s/veh]	9.67														
Intersection LOS	A														
Intersection V/C	0.275														

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0		
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00		
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000		
Crosswalk LOS	F			F			F			F		
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000		
c_b, Capacity of the bicycle lane [bicycles/h]	1674			1674			1674			1674		
d_b, Bicycle Delay [s]	0.48			0.48			0.48			0.48		
I_b,int, Bicycle LOS Score for Intersection	1.744			1.873			2.066			1.959		
Bicycle LOS	A			A			B			A		

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Enclave - Maplewood

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Scenario 10 PM 2024 Background

Report File: N:\...\PM Future Background.pdf

9/24/2022

**Turning Movement Volume: Detail**

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-T	Left	Thru		Right
1	County Road D and Southlawn Drive	Final Base	23	6	89	29	7	28	36	430	35	1	53	244	28	1009
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>23</b>	<b>6</b>	<b>90</b>	<b>29</b>	<b>7</b>	<b>28</b>	<b>36</b>	<b>434</b>	<b>35</b>	<b>1</b>	<b>54</b>	<b>246</b>	<b>28</b>	<b>1017</b>

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound					
			Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right
2	Beam Avenue and Southlawn Drive	Final Base	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>38</b>	<b>10</b>	<b>35</b>	<b>1</b>	<b>100</b>	<b>17</b>	<b>109</b>	<b>6</b>	<b>160</b>	<b>517</b>	<b>37</b>	<b>3</b>	<b>36</b>	<b>323</b>	<b>77</b>

Total Volume
1456
-
0
0
0
<b>1469</b>

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## Encalve - Maplewood

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Maplewood.vistro

Scenario 11 AM 2024 Future total

Report File: N:\...\AM 2024 Future total.pdf

9/24/2022

**Intersection Analysis Summary**

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	County Road D and Southlawn Drive	Signalized	HCM 7th Edition	NB Left	0.150	9.1	A
2	Beam Avenue and Southlawn Drive	Signalized	HCM 7th Edition	SB Left	0.142	7.4	A
3	Southlawn Drive and Project Driveway	Two-way stop	HCM 7th Edition	EB Left	0.003	9.7	A

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report**  
**Intersection 1: County Road D and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	9.1
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.150

**Intersection Setup**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	↵↑			↑↵			↵↑↑			↵↑↑		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	0	0	1	1	0	1	1	0	0
Entry Pocket Length [ft]	200.00	100.00	100.00	100.00	100.00	50.00	150.00	100.00	125.00	325.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Curb Present	No			No			No			No		
Crosswalk	No			No			No			No		

**Volumes**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D		
Base Volume Input [veh/h]	5	0	21	1	0	2	3	216	11	53	190	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00											
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	17	1	34	0	0	0	0	0	10	6	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	22	1	55	1	0	2	3	218	21	60	192	1
Peak Hour Factor	0.6250	1.0000	0.5830	0.2500	1.0000	0.5000	0.7500	0.9000	0.6880	0.7790	0.8800	0.2500
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	9	0	24	1	0	1	1	61	8	19	55	1
Total Analysis Volume [veh/h]	35	1	94	4	0	4	4	242	31	77	218	4
Presence of On-Street Parking	No		No	No		No	No		No	No		No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0			0		
v_di, Inbound Pedestrian Volume crossing m	0			0			0			0		
v_co, Outbound Pedestrian Volume crossing	0			0			0			0		
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0			0		
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	ProtPer	Permiss	Permiss	ProtPer	Permiss	Permiss
Signal Group	0	4	0	0	8	0	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-
Minimum Green [s]	0	7	0	0	7	0	5	15	0	5	15	0
Maximum Green [s]	0	15	0	0	15	0	15	60	0	15	60	0
Amber [s]	0.0	3.0	0.0	0.0	3.2	0.0	3.0	4.0	0.0	3.0	4.0	0.0
All red [s]	0.0	2.2	0.0	0.0	2.0	0.0	2.0	1.4	0.0	2.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	2.7	0.0	0.0	2.0	0.0	2.0	3.5	0.0	2.0	3.5	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	7	0
Pedestrian Clearance [s]	0	20	0	0	23	0	0	20	0	0	11	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No			No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	3.2	0.0	0.0	3.2	0.0	3.0	3.4	0.0	3.0	3.0	0.0
Minimum Recall		No			No		No	No		No	No	
Maximum Recall		No			No		No	No		No	No	
Pedestrian Recall		No			No		No	No		No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	C	R	L	C	R	L	C	C
C, Cycle Length [s]	40	40	40	40	40	40	40	40	40	40
L, Total Lost Time per Cycle [s]	5.20	5.20	5.20	5.20	5.40	5.40	5.40	5.00	5.00	5.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	3.20	3.20	3.20	3.20	0.00	3.40	3.40	0.00	3.00	3.00
g_i, Effective Green Time [s]	7	7	7	7	22	14	14	23	17	17
g / C, Green / Cycle	0.18	0.18	0.18	0.18	0.56	0.36	0.36	0.57	0.44	0.44
(v / s)_i Volume / Saturation Flow Rate	0.02	0.06	0.01	0.00	0.00	0.07	0.02	0.04	0.06	0.06
s, saturation flow rate [veh/h]	1412	1592	558	1589	1249	3560	1589	1781	1870	1858
c, Capacity [veh/h]	215	282	279	282	935	1281	572	1213	818	813
d1, Uniform Delay [s]	19.54	14.43	17.16	13.60	3.93	8.81	8.38	3.94	6.75	6.75
k, delay calibration	0.09	0.09	0.04	0.04	0.04	0.13	0.13	0.13	0.13	0.13
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.30	0.60	0.01	0.01	0.00	0.09	0.05	0.03	0.09	0.09
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.16	0.34	0.01	0.01	0.00	0.19	0.05	0.06	0.14	0.14
d, Delay for Lane Group [s/veh]	19.84	15.03	17.17	13.61	3.93	8.90	8.42	3.96	6.84	6.84
Lane Group LOS	B	B	B	B	A	A	A	A	A	A
Critical Lane Group	No	Yes	No	No	No	Yes	No	Yes	No	No
50th-Percentile Queue Length [veh/ln]	0.30	0.68	0.03	0.03	0.01	0.56	0.14	0.16	0.42	0.42
50th-Percentile Queue Length [ft/ln]	7.58	16.95	0.75	0.64	0.21	14.09	3.53	4.07	10.47	10.44
95th-Percentile Queue Length [veh/ln]	0.55	1.22	0.05	0.05	0.01	1.01	0.25	0.29	0.75	0.75
95th-Percentile Queue Length [ft/ln]	13.64	30.51	1.36	1.15	0.37	25.36	6.35	7.33	18.84	18.79

**Movement, Approach, & Intersection Results**

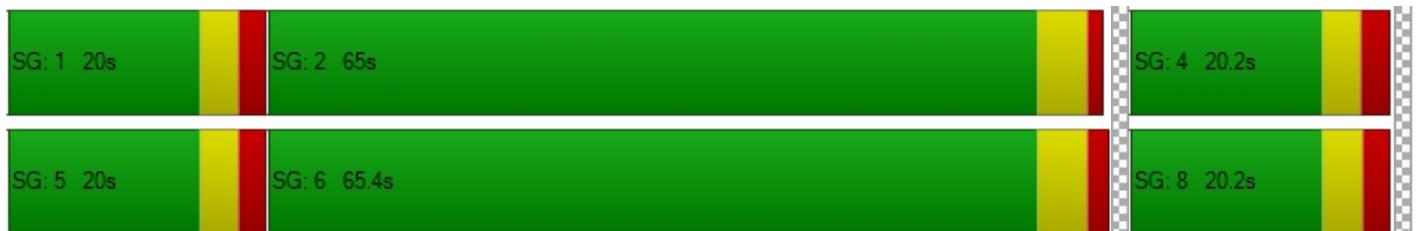
d_M, Delay for Movement [s/veh]	19.84	15.03	15.03	17.17	17.17	13.61	3.93	8.90	8.42	3.96	6.84	6.84
Movement LOS	B	B	B	B	B	B	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	16.32			15.39			8.77			6.10		
Approach LOS	B			B			A			A		
d_I, Intersection Delay [s/veh]	9.10											
Intersection LOS	A											
Intersection V/C	0.150											

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0		
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00		
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00		
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000		
Crosswalk LOS	F			F			F			F		
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000		
c_b, Capacity of the bicycle lane [bicycles/h]	751			751			3004			3004		
d_b, Bicycle Delay [s]	7.79			7.79			5.03			5.03		
I_b,int, Bicycle LOS Score for Intersection	1.774			1.573			1.788			1.806		
Bicycle LOS	A			A			A			A		

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**

**Intersection 2: Beam Avenue and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	7.4
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.142

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive			Beam Avenue				Beam Avenue			
Approach	Northbound			Southbound			Eastbound				Westbound			
Lane Configuration														
Turning Movement	Left	Thru	Right	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	1	1	0	0	1	1	0	0	1
Entry Pocket Length [ft]	125.00	100.00	100.00	225.00	100.00	275.00	325.0	100.0	100.0	225.0	325.0	100.0	100.0	125.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00				30.00			
Grade [%]	0.00			0.00			0.00				0.00			
Curb Present	No			No			No				No			
Crosswalk	No			No			No				No			

**Volumes**

Name	Southlawn Drive			Southlawn Drive			Beam Avenue				Beam Avenue			
	18	4	18	24	14	17	4	53	247	20	7	15	213	36
Base Volume Input [veh/h]	18	4	18	24	14	17	4	53	247	20	7	15	213	36
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00													
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	8	1	8	0	2	0	0	0	0	0	1
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	18	4	18	32	15	25	4	56	249	20	7	15	215	37
Peak Hour Factor	0.6430	0.2500	0.9000	0.7500	0.5830	0.7080	0.500	0.697	0.813	0.625	0.583	0.536	0.750	0.692
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	7	4	5	11	6	9	2	20	77	8	3	7	72	13
Total Analysis Volume [veh/h]	28	16	20	43	26	35	8	80	306	32	12	28	287	53
Presence of On-Street Parking	No		No	No		No	No			No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0				0			
v_di, Inbound Pedestrian Volume crossing m	0			0			0				0			
v_co, Outbound Pedestrian Volume crossing	0			0			0				0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0				0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0				0			
Bicycle Volume [bicycles/h]	0			0			0				0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Permi	ProtP	Permi	Permi	Permi	ProtP	Permi	Permi
Signal Group	0	8	0	0	4	0	0	5	2	0	0	1	6	0
Auxiliary Signal Groups														
Lead / Lag	-	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	10	0	0	10	0	0	5	10	0	0	5	10	0
Maximum Green [s]	0	30	0	0	30	0	0	30	30	0	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	0	5	0	0	5	0	0	0	5	0	0	0	5	0
Pedestrian Clearance [s]	0	10	0	0	10	0	0	0	10	0	0	0	10	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No	No			No	No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
Minimum Recall		No			No			No	No			No	No	
Maximum Recall		No			No			No	No			No	No	
Pedestrian Recall		No			No			No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	L	C	R	L	C	R	L	C	R
C, Cycle Length [s]	30	30	30	30	30	30	30	30	30	30	30
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00
g_i, Effective Green Time [s]	6	6	6	6	6	16	11	11	16	9	9
g / C, Green / Cycle	0.20	0.20	0.20	0.20	0.20	0.54	0.35	0.35	0.54	0.32	0.32
(v / s)_i Volume / Saturation Flow Rate	0.02	0.02	0.03	0.01	0.02	0.07	0.09	0.02	0.03	0.08	0.03
s, saturation flow rate [veh/h]	1341	1703	1372	1870	1589	1255	3560	1589	1202	3560	1589
c, Capacity [veh/h]	411	337	406	370	314	970	1262	564	941	1121	500
d1, Uniform Delay [s]	11.70	9.91	11.99	9.84	9.92	3.46	6.87	6.41	3.36	7.70	7.32
k, delay calibration	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.07	0.14	0.11	0.08	0.15	0.04	0.10	0.04	0.02	0.12	0.09
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.07	0.11	0.11	0.07	0.11	0.09	0.24	0.06	0.04	0.26	0.11
d, Delay for Lane Group [s/veh]	11.77	10.05	12.11	9.92	10.07	3.50	6.97	6.45	3.38	7.82	7.41
Lane Group LOS	B	B	B	A	B	A	A	A	A	A	A
Critical Lane Group	No	No	Yes	No	No	Yes	No	No	No	Yes	No
50th-Percentile Queue Length [veh/ln]	0.13	0.15	0.21	0.11	0.15	0.10	0.43	0.09	0.04	0.46	0.17
50th-Percentile Queue Length [ft/ln]	3.34	3.83	5.25	2.72	3.75	2.52	10.85	2.20	1.12	11.47	4.19
95th-Percentile Queue Length [veh/ln]	0.24	0.28	0.38	0.20	0.27	0.18	0.78	0.16	0.08	0.83	0.30
95th-Percentile Queue Length [ft/ln]	6.01	6.89	9.46	4.89	6.75	4.54	19.53	3.95	2.01	20.65	7.54

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	11.77	10.05	10.05	12.11	9.92	10.07	3.50	3.50	6.97	6.45	3.38	3.38	7.82	7.41
Movement LOS	B	B	B	B	A	B	A	A	A	A	A	A	A	A
d_A, Approach Delay [s/veh]	10.80			10.87			6.21			7.29				
Approach LOS	B			B			A			A				
d_I, Intersection Delay [s/veh]	7.43													
Intersection LOS	A													
Intersection V/C	0.142													

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000			
Crosswalk LOS	F			F			F			F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000			
c_b, Capacity of the bicycle lane [bicycles/h]	2000			2000			2000			2000			
d_b, Bicycle Delay [s]	0.00			0.00			0.00			0.00			
I_b,int, Bicycle LOS Score for Intersection	1.665			1.731			1.845			1.850			
Bicycle LOS	A			A			A			A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 3: Southlawn Drive and Project Driveway**

Control Type:	Two-way stop	Delay (sec / veh):	9.7
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.003

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive			Project Driveway			Project Driveway		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	⇌⇌			⇌⇌			⇌⇌			⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	1	0	0	1	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	100.00	0.00	0.00	100.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	No			No			No			No		

**Volumes**

Name	Southlawn Drive			Southlawn Drive			Project Driveway			Project Driveway		
Base Volume Input [veh/h]	4	26	0	0	64	5	2	0	2	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	3	16	0	0	0	0	0	17	0	52
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	4	26	3	16	65	5	2	0	2	17	0	52
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	7	1	4	18	1	1	0	1	5	0	14
Total Analysis Volume [veh/h]	4	28	3	17	71	5	2	0	2	18	0	57
Pedestrian Volume [ped/h]	0			0			0			0		

**Intersection Settings**

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			No	No
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			No	No
Number of Storage Spaces in Median	0	0	0	0

**Movement, Approach, & Intersection Results**

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.00	0.02	0.00	0.05
d_M, Delay for Movement [s/veh]	7.37	0.00	0.00	7.30	0.00	0.00	9.66	9.91	8.52	9.34	10.11	8.59
Movement LOS	A	A	A	A	A	A	A	A	A	A	B	A
95th-Percentile Queue Length [veh/ln]	0.01	0.00	0.00	0.03	0.01	0.00	0.01	0.01	0.01	0.07	0.17	0.17
95th-Percentile Queue Length [ft/ln]	0.17	0.08	0.00	0.71	0.36	0.00	0.19	0.15	0.15	1.63	4.26	4.26
d_A, Approach Delay [s/veh]	0.84			1.33			9.09			8.77		
Approach LOS	A			A			A			A		
d_I, Intersection Delay [s/veh]	4.09											
Intersection LOS	A											

Encalve - Maplewood

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Scenario 11 AM 2024 Future total

Report File: N:\...\AM 2024 Future total.pdf

9/24/2022

**Turning Movement Volume: Detail**

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
1	County Road D and Southlawn Drive	Final Base	5	0	21	1	0	2	3	216	11	53	190	1	503	
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	-	
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	
		Net New Trips	17	1	34	0	0	0	0	0	0	10	6	0	0	68
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>22</b>	<b>1</b>	<b>55</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>3</b>	<b>218</b>	<b>21</b>	<b>60</b>	<b>192</b>	<b>1</b>	<b>576</b>	

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound					
			Left	Thru	Right	Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right	
2	Beam Avenue and Southlawn Drive	Final Base	18	4	18	24	14	17	4	53	247	20	7	15	213	36	
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
		Net New Trips	0	0	0	8	1	8	0	2	0	0	0	0	0	0	1
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>18</b>	<b>4</b>	<b>18</b>	<b>32</b>	<b>15</b>	<b>25</b>	<b>4</b>	<b>56</b>	<b>249</b>	<b>20</b>	<b>7</b>	<b>15</b>	<b>215</b>	<b>37</b>	

Total Volume
690
-
0
20
0
<b>715</b>

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
3	Southlawn Drive and Project Driveway	Final Base	4	26	0	0	64	5	2	0	2	0	0	0	103
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	3	16	0	0	0	0	0	17	0	52	88
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>4</b>	<b>26</b>	<b>3</b>	<b>16</b>	<b>65</b>	<b>5</b>	<b>2</b>	<b>0</b>	<b>2</b>	<b>17</b>	<b>0</b>	<b>52</b>	<b>192</b>

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## Enclave - Maplewood

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Maplewood.vistro

Scenario 12 PM 2024 Future total

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9/24/2022

**Intersection Analysis Summary**

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	County Road D and Southlawn Drive	Signalized	HCM 7th Edition	NB Left	0.256	12.0	B
2	Beam Avenue and Southlawn Drive	Signalized	HCM 7th Edition	SB Left	0.281	9.7	A
3	Southlawn Drive and Project Driveway	Two-way stop	HCM 7th Edition	WB Left	0.015	11.1	B

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

**Intersection Level Of Service Report**  
**Intersection 1: County Road D and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	12.0
Analysis Method:	HCM 7th Edition	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.256

**Intersection Setup**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D			
Approach	Northbound			Southbound			Eastbound			Westbound			
Lane Configuration	↵↑			↑↵			↵↵↵			↵↑			
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	0	0	1	1	0	1	1	0	0	0
Entry Pocket Length [ft]	200.00	100.00	100.00	100.00	100.00	50.00	150.00	100.00	125.00	325.0	100.0	100.0	100.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00			
Grade [%]	0.00			0.00			0.00			0.00			
Curb Present	No			No			No			No			
Crosswalk	No			No			No			No			

**Volumes**

Name	Southlawn Drive			Best Buy Driveway			County Road D			County Road D			
Base Volume Input [veh/h]	23	6	89	29	7	28	36	430	35	1	53	244	28
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00												
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.010	1.010	1.010	1.010
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	9	1	19	0	1	0	0	0	29	0	17	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	32	7	109	29	8	28	36	434	64	1	71	246	28
Peak Hour Factor	0.5750	0.5000	0.8560	0.9060	0.4380	0.8750	0.8180	0.8810	0.6730	0.250	0.722	0.924	0.778
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	14	4	32	8	5	8	11	123	24	1	25	67	9
Total Analysis Volume [veh/h]	56	14	127	32	18	32	44	493	95	4	98	266	36
Presence of On-Street Parking	No		No	No		No	No		No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0			0			0			
v_di, Inbound Pedestrian Volume crossing m	0			0			0			0			
v_co, Outbound Pedestrian Volume crossing	0			0			0			0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0			0			0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0			0			0			
Bicycle Volume [bicycles/h]	0			0			0			0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	60
Coordination Type	Time of Day Pattern Coordinated
Actuation Type	Semi-actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	ProtPer	Permiss	Permiss	Permi	ProtP	Permi	Permi
Signal Group	0	4	0	0	8	0	1	6	0	0	5	2	0
Auxiliary Signal Groups													
Lead / Lag	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	7	0	0	7	0	5	15	0	0	5	15	0
Maximum Green [s]	0	15	0	0	15	0	15	60	0	0	15	60	0
Amber [s]	0.0	3.0	0.0	0.0	3.2	0.0	3.0	4.0	0.0	0.0	3.0	4.0	0.0
All red [s]	0.0	2.2	0.0	0.0	2.0	0.0	2.0	1.4	0.0	0.0	2.0	1.0	0.0
Split [s]	0	18	0	0	18	0	12	29	0	0	13	30	0
Vehicle Extension [s]	0.0	2.7	0.0	0.0	2.0	0.0	2.0	3.5	0.0	0.0	2.0	3.5	0.0
Walk [s]	0	7	0	0	7	0	0	7	0	0	0	7	0
Pedestrian Clearance [s]	0	20	0	0	23	0	0	20	0	0	0	11	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No			No			No				No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	2.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	3.2	0.0	0.0	3.2	0.0	3.0	3.4	0.0	0.0	3.0	3.0	0.0
Minimum Recall		No			No		No	No			No	No	
Maximum Recall		No			No		No	No			No	No	
Pedestrian Recall		No			No		No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	C	R	L	C	R	L	C	C
C, Cycle Length [s]	60	60	60	60	60	60	60	60	60	60
L, Total Lost Time per Cycle [s]	5.20	5.20	5.20	5.20	5.40	5.40	5.40	5.00	5.00	5.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	3.20	3.20	3.20	3.20	0.00	3.40	3.40	0.00	3.00	3.00
g_i, Effective Green Time [s]	13	13	13	13	37	28	28	37	29	29
g / C, Green / Cycle	0.21	0.21	0.21	0.21	0.61	0.46	0.46	0.62	0.49	0.49
(v / s)_i Volume / Saturation Flow Rate	0.04	0.09	0.06	0.02	0.04	0.14	0.06	0.06	0.08	0.08
s, saturation flow rate [veh/h]	1355	1614	817	1589	1185	3560	1589	1781	1870	1793
c, Capacity [veh/h]	184	341	271	336	860	1640	732	1102	920	882
d1, Uniform Delay [s]	28.40	20.45	20.10	19.05	4.68	10.13	9.28	4.62	8.43	8.44
k, delay calibration	0.09	0.09	0.04	0.04	0.04	0.50	0.50	0.50	0.50	0.50
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.79	0.69	0.12	0.05	0.01	0.47	0.37	0.17	0.39	0.41
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.30	0.41	0.18	0.10	0.05	0.30	0.13	0.09	0.17	0.17
d, Delay for Lane Group [s/veh]	29.18	21.14	20.22	19.10	4.69	10.60	9.65	4.79	8.82	8.86
Lane Group LOS	C	C	C	B	A	B	A	A	A	A
Critical Lane Group	No	Yes	No	No	No	Yes	No	Yes	No	No
50th-Percentile Queue Length [veh/ln]	0.80	1.65	0.54	0.34	0.16	1.86	0.69	0.43	1.03	1.01
50th-Percentile Queue Length [ft/ln]	19.95	41.20	13.38	8.47	4.11	46.48	17.33	10.71	25.83	25.26
95th-Percentile Queue Length [veh/ln]	1.44	2.97	0.96	0.61	0.30	3.35	1.25	0.77	1.86	1.82
95th-Percentile Queue Length [ft/ln]	35.91	74.16	24.09	15.25	7.40	83.66	31.19	19.27	46.49	45.48

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	29.18	21.14	21.14	20.22	20.22	19.10	4.69	10.60	9.65	4.79	4.79	8.84	8.86
Movement LOS	C	C	C	C	C	B	A	B	A	A	A	A	A
d_A, Approach Delay [s/veh]	23.43			19.78			10.05			7.82			
Approach LOS	C			B			B			A			
d_I, Intersection Delay [s/veh]	11.97												
Intersection LOS	B												
Intersection V/C	0.256												

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0			0.0			0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00			0.00			0.00			
d_p, Pedestrian Delay [s]	0.00			0.00			0.00			0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000			0.000			0.000			
Crosswalk LOS	F			F			F			F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000			2000			2000			
c_b, Capacity of the bicycle lane [bicycles/h]	427			427			787			833			
d_b, Bicycle Delay [s]	18.57			18.57			11.04			10.21			
I_b,int, Bicycle LOS Score for Intersection	1.885			1.695			2.081			1.890			
Bicycle LOS	A			A			B			A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 2: Beam Avenue and Southlawn Drive**

Control Type:	Signalized	Delay (sec / veh):	9.7
Analysis Method:	HCM 7th Edition	Level Of Service:	A
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.281

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive				Beam Avenue				Beam Avenue			
Approach	Northbound			Southbound				Eastbound				Westbound			
Lane Configuration	T T			T L R				T L R				T L R			
Turning Movement	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right	U-tu	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	1	0	0	1	1	0	0	1
Entry Pocket Length [ft]	125.00	100.00	100.00	225.0	100.0	100.0	275.0	325.0	100.0	100.0	225.0	325.0	100.0	100.0	125.0
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00				30.00				30.00			
Grade [%]	0.00			0.00				0.00				0.00			
Curb Present	No			No				No				No			
Crosswalk	No			No				No				No			

**Volumes**

Name	Southlawn Drive			Southlawn Drive				Beam Avenue				Beam Avenue			
Base Volume Input [veh/h]	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Proportion of CAVs [%]	0.00														
Growth Factor	1.0100	1.0100	1.0100	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010	1.010
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	1	0	0	4	0	4	0	6	0	0	0	0	0	3
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Right Turn on Red Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	38	11	35	1	104	17	113	6	166	517	37	3	36	323	80
Peak Hour Factor	0.7920	0.4170	0.8750	0.250	0.884	0.607	0.692	0.750	0.859	0.934	0.712	0.750	0.750	0.879	0.679
Other Adjustment Factor	1.0000	1.0000	1.0000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000
Total 15-Minute Volume [veh/h]	12	7	10	1	29	7	41	2	48	138	13	1	12	92	29
Total Analysis Volume [veh/h]	48	26	40	4	118	28	163	8	193	554	52	4	48	367	118
Presence of On-Street Parking	No		No	No			No	No			No	No			No
On-Street Parking Maneuver Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Local Bus Stopping Rate [/h]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
v_do, Outbound Pedestrian Volume crossing	0			0				0				0			
v_di, Inbound Pedestrian Volume crossing m	0			0				0				0			
v_co, Outbound Pedestrian Volume crossing	0			0				0				0			
v_ci, Inbound Pedestrian Volume crossing mi	0			0				0				0			
v_ab, Corner Pedestrian Volume [ped/h]	0			0				0				0			
Bicycle Volume [bicycles/h]	0			0				0				0			

**Intersection Settings**

Located in CBD	No
Signal Coordination Group	-
Cycle Length [s]	90
Coordination Type	Free Running
Actuation Type	Fully actuated
Offset [s]	0.0
Offset Reference	Lead Green - Beginning of First Green
Permissive Mode	SingleBand
Lost time [s]	0.00

**Phasing & Timing**

Control Type	Permiss	Permiss	Permiss	Permi	Permi	Permi	Permi	Permi	ProtP	Permi	Permi	Permi	ProtP	Permi	Permi
Signal Group	0	8	0	0	0	4	0	0	5	2	0	0	1	6	0
Auxiliary Signal Groups															
Lead / Lag	-	-	-	-	-	-	-	-	Lead	-	-	-	Lead	-	-
Minimum Green [s]	0	10	0	0	0	10	0	0	5	10	0	0	5	10	0
Maximum Green [s]	0	30	0	0	0	30	0	0	30	30	0	0	30	30	0
Amber [s]	0.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
All red [s]	0.0	1.0	0.0	0.0	0.0	1.0	0.0	0.0	1.0	1.0	0.0	0.0	1.0	1.0	0.0
Split [s]	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Extension [s]	0.0	3.0	0.0	0.0	0.0	3.0	0.0	0.0	3.0	3.0	0.0	0.0	3.0	3.0	0.0
Walk [s]	0	5	0	0	0	5	0	0	0	5	0	0	0	5	0
Pedestrian Clearance [s]	0	10	0	0	0	10	0	0	0	10	0	0	0	10	0
Delayed Vehicle Green [s]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Rest In Walk		No				No				No				No	
I1, Start-Up Lost Time [s]	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
I2, Clearance Lost Time [s]	0.0	2.0	0.0	0.0	0.0	2.0	0.0	0.0	2.0	2.0	0.0	0.0	2.0	2.0	0.0
Minimum Recall		No				No			No	No			No	No	
Maximum Recall		No				No			No	No			No	No	
Pedestrian Recall		No				No			No	No			No	No	
Detector Location [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector Length [ft]	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
I, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Exclusive Pedestrian Phase**

Pedestrian Signal Group	0
Pedestrian Walk [s]	0
Pedestrian Clearance [s]	0

**Lane Group Calculations**

Lane Group	L	C	L	C	R	L	C	R	L	C	R
C, Cycle Length [s]	36	36	36	36	36	36	36	36	36	36	36
L, Total Lost Time per Cycle [s]	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00	4.00
l1_p, Permitted Start-Up Lost Time [s]	2.00	0.00	2.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
l2, Clearance Lost Time [s]	2.00	2.00	2.00	2.00	2.00	0.00	2.00	2.00	0.00	2.00	2.00
g_i, Effective Green Time [s]	10	10	10	10	10	18	12	12	18	10	10
g / C, Green / Cycle	0.27	0.27	0.27	0.27	0.27	0.51	0.34	0.34	0.51	0.28	0.28
(v / s)_i Volume / Saturation Flow Rate	0.04	0.04	0.09	0.01	0.10	0.17	0.16	0.03	0.05	0.10	0.07
s, saturation flow rate [veh/h]	1192	1689	1335	1870	1589	1215	3560	1589	1031	3560	1589
c, Capacity [veh/h]	446	452	451	500	425	845	1212	541	726	985	440
d1, Uniform Delay [s]	11.82	10.07	13.09	9.82	10.78	5.02	9.29	8.11	4.73	10.52	10.19
k, delay calibration	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11	0.11
l, Upstream Filtering Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
d2, Incremental Delay [s]	0.11	0.15	0.32	0.05	0.57	0.14	0.27	0.08	0.04	0.23	0.32
d3, Initial Queue Delay [s]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Rp, platoon ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PF, progression factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

**Lane Group Results**

X, volume / capacity	0.11	0.15	0.27	0.06	0.38	0.24	0.46	0.10	0.07	0.37	0.27
d, Delay for Lane Group [s/veh]	11.92	10.21	13.41	9.87	11.35	5.16	9.56	8.19	4.78	10.75	10.52
Lane Group LOS	B	B	B	A	B	A	A	A	A	B	B
Critical Lane Group	No	No	No	No	Yes	Yes	No	No	No	Yes	No
50th-Percentile Queue Length [veh/ln]	0.27	0.32	0.74	0.13	0.87	0.49	1.26	0.21	0.12	0.92	0.59
50th-Percentile Queue Length [ft/ln]	6.63	8.07	18.55	3.31	21.80	12.21	31.46	5.26	2.90	22.98	14.81
95th-Percentile Queue Length [veh/ln]	0.48	0.58	1.34	0.24	1.57	0.88	2.27	0.38	0.21	1.65	1.07
95th-Percentile Queue Length [ft/ln]	11.93	14.53	33.40	5.96	39.23	21.98	56.63	9.46	5.21	41.36	26.65

**Movement, Approach, & Intersection Results**

d_M, Delay for Movement [s/veh]	11.92	10.21	10.21	13.41	13.41	9.87	11.35	5.16	5.16	9.56	8.19	4.78	4.78	10.75	10.52
Movement LOS	B	B	B	B	B	A	B	A	A	A	A	A	A	B	B
d_A, Approach Delay [s/veh]	10.93			12.02				8.38				10.12			
Approach LOS	B			B				A				B			
d_I, Intersection Delay [s/veh]	9.72														
Intersection LOS	A														
Intersection V/C	0.281														

**Other Modes**

g_Walk,mi, Effective Walk Time [s]	0.0			0.0				0.0				0.0			
M_corner, Corner Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00				0.00				0.00			
M_CW, Crosswalk Circulation Area [ft <sup>2</sup> /ped]	0.00			0.00				0.00				0.00			
d_p, Pedestrian Delay [s]	0.00			0.00				0.00				0.00			
I_p,int, Pedestrian LOS Score for Intersection	0.000			0.000				0.000				0.000			
Crosswalk LOS	F			F				F				F			
s_b, Saturation Flow Rate of the bicycle lane	2000			2000				2000				2000			
c_b, Capacity of the bicycle lane [bicycles/h]	1669			1669				1669				1669			
d_b, Bicycle Delay [s]	0.49			0.49				0.49				0.49			
I_b,int, Bicycle LOS Score for Intersection	1.748			1.881				2.066				1.963			
Bicycle LOS	A			A				B				A			

**Sequence**

Ring 1	1	2	-	4	-	-	-	-	-	-	-	-	-	-	-	-
Ring 2	5	6	-	8	-	-	-	-	-	-	-	-	-	-	-	-
Ring 3	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Ring 4	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



**Intersection Level Of Service Report**  
**Intersection 3: Southlawn Drive and Project Driveway**

Control Type:	Two-way stop	Delay (sec / veh):	11.1
Analysis Method:	HCM 7th Edition	Level Of Service:	B
Analysis Period:	15 minutes	Volume to Capacity (v/c):	0.015

**Intersection Setup**

Name	Southlawn Drive			Southlawn Drive			Project Driveway			Project Driveway		
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration	⇌⇌			⇌⇌			⇌⇌			⇌⇌		
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	1	0	0	1	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	100.00	0.00	0.00	100.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	No			No			No			No		

**Volumes**

Name	Southlawn Drive			Southlawn Drive			Project Driveway			Project Driveway		
Base Volume Input [veh/h]	5	118	0	0	94	6	6	0	7	0	0	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100	1.0100
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	10	47	0	0	0	0	0	8	0	29
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	5	119	10	47	95	6	6	0	7	8	0	29
Peak Hour Factor	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200	0.9200
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	1	32	3	13	26	2	2	0	2	2	0	8
Total Analysis Volume [veh/h]	5	129	11	51	103	7	7	0	8	9	0	32
Pedestrian Volume [ped/h]	0			0			0			0		

**Intersection Settings**

Priority Scheme	Free	Free	Stop	Stop
Flared Lane			No	No
Storage Area [veh]	0	0	0	0
Two-Stage Gap Acceptance			No	No
Number of Storage Spaces in Median	0	0	0	0

**Movement, Approach, & Intersection Results**

V/C, Movement V/C Ratio	0.00	0.00	0.00	0.04	0.00	0.00	0.01	0.00	0.01	0.01	0.00	0.03
d_M, Delay for Movement [s/veh]	7.44	0.00	0.00	7.56	0.00	0.00	11.06	11.65	8.63	11.08	11.73	8.80
Movement LOS	A	A	A	A	A	A	B	B	A	B	B	A
95th-Percentile Queue Length [veh/ln]	0.01	0.00	0.00	0.09	0.04	0.00	0.04	0.02	0.02	0.05	0.10	0.10
95th-Percentile Queue Length [ft/ln]	0.21	0.10	0.00	2.19	1.09	0.00	0.88	0.60	0.60	1.14	2.53	2.53
d_A, Approach Delay [s/veh]	0.26			2.39			9.76			9.30		
Approach LOS	A			A			A			A		
d_I, Intersection Delay [s/veh]	2.63											
Intersection LOS	B											

Enclave - Maplewood

Vistro File: N:\...\220916\_R0037066.00\_Enclave - Maplewood.vistro

Scenario 12 PM 2024 Future total

Report File: N:\...\PM 2024 Future total.pdf

9/24/2022

**Turning Movement Volume: Detail**

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume	
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	U-T	Left	Thru		Right
1	County Road D and Southlawn Drive	Final Base	23	6	89	29	7	28	36	430	35	1	53	244	28	1009
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	9	1	19	0	1	0	0	0	0	29	0	17	0	76
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>32</b>	<b>7</b>	<b>109</b>	<b>29</b>	<b>8</b>	<b>28</b>	<b>36</b>	<b>434</b>	<b>64</b>	<b>1</b>	<b>71</b>	<b>246</b>	<b>28</b>	<b>1093</b>

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound					
			Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right	U-T	Left	Thru	Right
2	Beam Avenue and Southlawn Drive	Final Base	38	10	35	1	99	17	108	6	158	512	37	3	36	320	76
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	1	0	0	4	0	4	0	6	0	0	0	0	0	3
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>38</b>	<b>11</b>	<b>35</b>	<b>1</b>	<b>104</b>	<b>17</b>	<b>113</b>	<b>6</b>	<b>166</b>	<b>517</b>	<b>37</b>	<b>3</b>	<b>36</b>	<b>323</b>	<b>80</b>

Total Volume
1456
-
0
18
0
<b>1487</b>

ID	Intersection Name	Volume Type	Northbound			Southbound			Eastbound			Westbound			Total Volume
			Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
3	Southlawn Drive and Project Driveway	Final Base	5	118	0	0	94	6	6	0	7	0	0	0	236
		Growth Factor	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	1.01	-
		In Process	0	0	0	0	0	0	0	0	0	0	0	0	0
		Net New Trips	0	0	10	47	0	0	0	0	0	8	0	29	94
		Other	0	0	0	0	0	0	0	0	0	0	0	0	0
		<b>Future Total</b>	<b>5</b>	<b>119</b>	<b>10</b>	<b>47</b>	<b>95</b>	<b>6</b>	<b>6</b>	<b>0</b>	<b>7</b>	<b>8</b>	<b>0</b>	<b>29</b>	<b>332</b>

**APPENDIX E**  
**Crash Data**  
**MN CMAT Training Data Dictionary**

CRASH_MONTH	CRASH_DAY	CRASH_YEAR	CRASHSEVERITY	NUMBERKILLED	MANNEROFCOLLISION	FIRSTHARMFULEVENT	RELATIONTOINTERSECTION	ROADWAY_NAME	INTERSECTION_NAME	BASIC_TYPE
6	7	2017	3	0	5	10	3	SOUTHLAWN DR		10
12	28	2018	4	0	5	10	3	SOUTHLAWN DR		9
3	4	2019	1	1		8	3	E COUNTY RD D		1
10	31	2021	5	0	90	10	3	SOUTHLAWN DR	COUNTY ROAD D	90

CRASH_MONTH	CRASH_DAY	CRASH_YEAR	CRASHSEVERITY	NUMBERKILLED	MANNEROFCOLLISION	FIRSTHARMFULEVENT	RELATIONTOINTERSECTION	ROADWAY_NAME	INTERSECTION_NAME	BASIC_TYPE
11	10	2017	5	0	13	10	3	BEAM AVE E	SOUTHLAWN DR	9
12	2	2017	5	0	5	10	3	SOUTHLAWN DR		10
7	2	2017	5	0	90	10	3	SOUTHLAWN DR		90
9	27	2017	4	0	5	10	3	SOUTHLAWN DR		9
12	21	2017	4	0	90	10	3	SOUTHLAWN DR		90
3	22	2017	5	0		11	90	SOUTHLAWN DR		4
9	12	2018	5	0	5	10	3	BEAM AVE E		10
3	29	2019	4	0	5	10	3	BEAM AVE E		10
11	25	2019	2	0	5	10	3	BEAM AVE E		10
8	21	2019	3	0	5	10	3	BEAM AVE E		10
9	27	2019	3	0	5	10	3	BEAM AVE E		10
5	31	2019	4	0	5	10	10	BEAM AVE E		10
10	13	2019	4	0	5	10	3	BEAM AVE E	SOUTHLAWN DR	10
12	11	2019	4	0	5	10	3	SOUTHLAWN DR		10
6	1	2019	5	0	13	10	3	SOUTHLAWN DR	BEAM AVE E	9
5	31	2019	4	0	5	10	3	SOUTHLAWN DR		10
7	11	2019	4	0	5	10	3	SOUTHLAWN DR		9
8	27	2020	3	0	5	10	3	BEAM AVE E		10
2	17	2020	3	0	5	10	3	BEAM AVE E		10
9	17	2020	3	0	13	10	3	SOUTHLAWN DR		7
5	6	2021	3	0	12	10	3	BEAM AVE E		7
1	31	2021	4	0	5	10	3	SOUTHLAWN DR		10
3	21	2021	4	0	12	10	10	SOUTHLAWN DR		7
1	12	2021	3	0	12	10	3	RAMP67		7
1	3	2022	4	0	5	10	10	SOUTHLAWN DR	BEAM AVE E	10
1	12	2022	4	0	12	10	27	RAMP68	BEAM AVE E	7

# MnCMAT2 Data Dictionary

This file contains information on all of the Data Elements and associated Attributes in MnCMAT2.

The table below lists all the Data Elements. They are in order first by level, then alphabetically.

**APPCODEID** and **APPCODENAME** (columns A and B) come from the MnDOT source data file. They are used only for reference here.

**ABBRV** (column C) is the abbreviated column heading used in some reports.

**Final Name** (column D) lists the name of the data element.

**Level** (Column E) shows whether the data element is at the Crash, Spatial, or Unit Level

**Filter** (column F) indicates if the data element has a Filter list. (for example, Incident ID does NOT have a Filter list as you would have to list all Incident ID's in the dropdown menu).

**Chart** (column G) indicates whether a Chart is available for that data element.

APPCODEID	APPCODENAME	ABBRV	Final Name	LEVEL	FILTER	CHART
		BASIC	Basic Type	Crash Level	YES	YES
	CITY	CITY	City	Crash Level	YES	YES
	COUNTY	CNTY	County	Crash Level	YES	YES
1	Crash Severity	CSEV	Crash Severity	Crash Level	YES	YES
114	Accident Day	DAYW	Day of Week	Crash Level	YES	YES
	Date	DATE	Date	Crash Level	YES	
102	Divided Roadway Direction	DIVDIR	Divided Rdwy Dir	Crash Level	YES	
69	First Harmful Event/Crash Type	FHE	First Harmful Event	Crash Level	YES	YES
	INCIDENT NUMBER	INCID	Incident ID	Crash Level		
	INTERSECTION WITH	INTW	Intersection Name	Crash Level		
29	Light Condition	LIGHT	Light Condition	Crash Level	YES	YES
	LOCAL ID	LOCALID	Local ID	Crash Level		
14	Manner of Collision	MOC	Manner of Collision	Crash Level	YES	YES
	Measure	MEAS	Measure	Crash Level		
115	Accident Month	MON	Month	Crash Level	YES	YES
	Narrative	Narrative	Narrative	Crash Level		
	NUMBER OF FATALITIES	NUMK	Number Killed	Crash Level	YES	YES
	NUMBER OF VEHICLES	NUMV	Number of Vehicles	Crash Level	YES	YES
	Officer Sketch		Officer Sketch	Crash Level		
28	Relationship to Intersection	RELINT	Relationship to Intersection	Crash Level	YES	YES
45	Roadway Surface	RDSRF	Roadway Surface	Crash Level	YES	YES
	ROUTE NAME	RNAME	Route Name	Crash Level		
	ROUTE NUMBER	RNUM	Route Number	Crash Level		
48	Route System	RSYS	Route System	Crash Level	YES	YES
	TIME OF CRASH	TIME	Time	Crash Level	YES	YES
	Township	TWNS	Township	Crash Level		
63	Weather Type	WTHR1	Weather 1	Crash Level	YES	YES
63	Weather Type	WTHR2	Weather 2	Crash Level	YES	YES
46	Roadway Workzone	WZ	Work Zone Type	Crash Level	YES	YES
	YEAR	YEAR	Year	Crash Level	YES	YES
	CITY		City	SPATIAL	YES	
	COUNTY		County	SPATIAL	YES	

	Latitude		Latitude	SPATIAL		
	Longitude		Longitude	SPATIAL		
	MNDOT DISTRICT	DIST	MNDOT District	SPATIAL	YES	YES
	STATE PATROL DISTRICT	PTRLD	State Patrol District	SPATIAL	YES	YES
	TRIBAL GOVERNMENT	TRIBE	Tribal Government	SPATIAL	YES	YES
	UTMX	UTMX	UTMX	SPATIAL		
	UTMY	UTMY	UTMY	SPATIAL		
	AGE	AGE	Age	Unit 0-8	YES	YES
76	Horizontal Alignment	ALGN	Alignment	Unit 0-4	YES	YES
10	Contributing Factor/Action Time of Crash	CFACT1	Contrib Factor1	Unit 0-8	YES	
10	Contributing Factor/Action Time of Crash	CFACT2	Contrib Factor2	Unit 0-8	YES	
15	Direction of Travel	DIR	Direction of Travel	Unit 0-4	YES	
77	Grade	GRADE	Grade	Unit 0-4	YES	YES
112	Non-Motorist Location at Time of Crash	NMLOC	Non-Motorist Location	Unit 5-8	YES	
83	Non-Motorist Maneuver Prior to Crash	NMMAN	Non-Motorist Maneuver	Unit 5-8	YES	
39	Driver Physical Condition	PCOND	Physical Condition	Unit 0-8	YES	YES
43	Roadway Design	RDSGN	Roadway Design	Unit 0-4	YES	YES
52	Sex Code	SEX	Sex	Unit 0-8	YES	YES
118	Speed Limit MPH	SL	Speed Limit	Unit 0-4	YES	YES
55	Traffic Control Device	TCD	Traffic Control Device	Unit 0-4	YES	YES
72	Unit Type	UTYPE	Unit Type	Unit 0-8	YES	YES
3	Pre-Crash Maneuver	VMAN	Vehicle Maneuver	Unit 0-4	YES	
59	Vehicle Type	VTYPE	Vehicle Type	Unit 0-4	YES	YES

The following 3 tabs contain the data elements and attributes.

**Filter and Charts** shows the attributes and the conversion tables used in both Filters and Charts.

**Contributing Factor** shows attributes. It has its own tab due to the large number of attributes.

**First Harmful Event** shows attributes. It has its own tab due to the large number of attributes.

The left hand columns (**columns A through D**) contain all data elements and attribute names and values.

Values in Red are 'retired'. These are attributes in OLD (pre-2016) crashes that did not match with the NEW attributes.

The right hand columns (**columns F through S**) contain the attributes used in Charts and Filter lists.

Columns I through F show the values that apply for each attribute.

These two sets of columns are normally the same. Examples of when and why they are different:

**Too Many Attributes:** For example - **Age**

The actual age (in years) is stored for each person. But a Filter list or Chart of more than 100 numbers is unmanageable, so ages are grouped into categories

**Old Attributes:** For Example - **Direction of Travel**

OLD crashes had 8 directions. NEW crashes have only the 4 cardinal directions

So OLD crashes with Direction of NORTHEAST retain the value of 5, but **when you Chart and Filter:**

NORTHEAST is grouped into the Northbound category. Likewise, NORTHWEST is put into Northbound, and SOUTHEAST and SOUTHWEST are put into Southbound (see example below).

ACTUAL VALUES		Filter and Chart Values			
Northbound	1	Northbound	1	5	8
Southbound	2	Southbound	2	6	7
Eastbound	3	Eastbound	3		
Westbound	4	Westbound	4		
Not on Roadway	10	Not on Roadway	10		
Unknown	99	Unknown	99		
	0				
NORTHEAST	5				
SOUTHEAST	6				
SOUTHWEST	7				
NORTHWEST	8				
OTHER	90				
Not Applicable	98				

Please see the "Introduction" tab for more information on this tab

Actual Attributes>>			
APPCODEID	APPCODENAME	NAME	VALUE
	Basic Type	Pedestrian	1
	Basic Type	Bike	2
	Basic Type	Single Veh ROR	3
	Basic Type	Single Veh other	4
	Basic Type	Sideswipe Passing	5
	Basic Type	Sideswipe Opposing	6
	Basic Type	Rear End	7
	Basic Type	Head On	8
	Basic Type	Left Turn	9
	Basic Type	Angle	10
	Basic Type	Other	99
<b>City</b>			
<b>County</b>			
1	Crash Severity		0
1	Crash Severity	K -Fatal	1
1	Crash Severity	A- Serious Injury	2
1	Crash Severity	B - Minor Injury	3
1	Crash Severity	C - Possible Injury	4
1	Crash Severity	N - Prop Dam Only	5
114	Day Of Week		0
114	Day Of Week	Sunday	1
114	Day Of Week	Monday	2
114	Day Of Week	Tuesday	3
114	Day Of Week	Wednesday	4
114	Day Of Week	Thursday	5
114	Day Of Week	Friday	6
114	Day Of Week	Saturday	7
114	Day Of Week		
<b>Date</b>			
44	Divided Rdwy Dir	Not Applicable	98
44	Divided Rdwy Dir	East	E
44	Divided Rdwy Dir	North	N
44	Divided Rdwy Dir	South	S
44	Divided Rdwy Dir	West	W
44	Divided Rdwy Dir		0
69	First Harmful Event	See separate Tab	
<b>Incident ID</b>			
<b>Intersect With</b>			
29	Light Condition	Daylight	1

FILTER AND CHART DISPLAYS>>>			
APPCODEID	Final Name	Attributes	Values>>>>>
	Basic Type	Pedestrian	1
	Basic Type	Bike	2
	Basic Type	Single Veh ROR	3
	Basic Type	Single Veh other	4
	Basic Type	Sideswipe Same Direction	5
	Basic Type	Sideswipe Opposing Direction	6
	Basic Type	Rear End	7
	Basic Type	Head On	8
	Basic Type	Left Turn	9
	Basic Type	Angle	10
	Basic Type	Other	99
<b>City</b>			
<b>County</b>			
1	Crash Severity		0
1	Crash Severity	K -Fatal	1
1	Crash Severity	A- Serious Injury	2
1	Crash Severity	B - Minor Injury	3
1	Crash Severity	C - Possible Injury	4
1	Crash Severity	N - Prop Dam Only	5
114	Day Of Week		0
114	Day Of Week	Sunday	1
114	Day Of Week	Monday	2
114	Day Of Week	Tuesday	3
114	Day Of Week	Wednesday	4
114	Day Of Week	Thursday	5
114	Day Of Week	Friday	6
114	Day Of Week	Saturday	7
114	Day Of Week		
<b>Date</b>			
44	Divided Rdwy Dir	Not Applicable	98
44	Divided Rdwy Dir	East	E
44	Divided Rdwy Dir	North	N
44	Divided Rdwy Dir	South	S
44	Divided Rdwy Dir	West	W
44	Divided Rdwy Dir		0
<b>Incident ID</b>			
<b>Intersect With</b>			
29	Light Condition	Daylight	1





48	Route System	Township Road	8
48	Route System	Unorganized Township Road	9
48	Route System	Local (or City) Street - MUN	10
48	Route System	National Park Road - NATP	11
48	Route System	National Forest Road - NATF	12
48	Route System	Indian Service Road - IND	13
48	Route System	State Forest Road - SFR	14
48	Route System	State Park Road - SPRK	15
48	Route System	Military Road - MIL	16
48	Route System	National Monument Road - NATM	17
48	Route System	National Wildlife Refuge Road	18
48	Route System	Frontage Road - FRNT	19
48	Route System	State Game Preserve Road	20
48	Route System	Private Road Open to Public	21
48	Route System	Ramp or Connector	22
48	Route System	Airport Roads	23
48	Route System	Bureau of Indian Affairs Road - BIA	24
48	Route System	Non-Trafficway	25
48	Route System	Other Local Road - OLR	26
48	Route System	Railroad Service Road - RSR	27
48	Route System	State Toll Road - STL	28
48	Route System	Local Toll Road - LTL	29
48	Route System	Alleyways	30
48	Route System	USBR Road - BRR	31
48	Route System	Other Road	32
48	Route System	BLM Road	33
48	Route System	Non Trafficway	34
48	Route System	HOV/HOT/Reversible Lanes on Interstate - IH	41
48	Route System	UHO HOV/HOT/Reversible Lanes on US Hwy	42
48	Route System	MHO HOV/HOT/Reversible Lanes on MN Hwy	43
48	Route System	Non-numbered Interstates - UNI	51
48	Route System	Non-numbered US Hwy - UNU	52
48	Route System	Non-numbered MN Hwy - UNM	53
48	Route System	Not Located	98

48	Route System	08-TWNS	8
48	Route System	09-UTWN	9
48	Route System	10-MUN	10
48	Route System	11-NATP	11
48	Route System	12-NATF	12
48	Route System	13-IND	13
48	Route System	14-SFR	14
48	Route System	15-SPRK	15
48	Route System	16-MIL	16
48	Route System	17-NATM	17
48	Route System	18-NWRR	18
48	Route System	19-FRNT	19
48	Route System	20-SGPR	20
48	Route System	21-PRIV	21
48	Route System	22-RAMP	22
48	Route System	23-AIRP	23
48	Route System	24-BIA	24
48	Route System	25-NTRF	25
48	Route System	26-OLR	26
48	Route System	27-RSR	27
48	Route System	28-STL	28
48	Route System	29-LTL	29
48	Route System	30-ALLY	30
48	Route System	31-BRR	31
48	Route System	32-OTHR	32
48	Route System	33-BLM	33
48	Route System	34-NTRF	34
48	Route System	41-IHO	41
48	Route System	42-UHO	42
48	Route System	43-MHO	43
48	Route System	51-UNI	51
48	Route System	52-UNU	52
48	Route System	53-UNM	53
48	Route System	UNK	98

	Time	ACTUAL TIME	

	Time	12:00 AM TO 12:59 AM	
		1:00 AM TO 1:59 AM	
		ETC	

	Township		
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	Township		
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63	Weather Type	Clear	1
63	Weather Type	Cloudy	2
63	Weather Type	Rain	3
63	Weather Type	Snow	4
63	Weather Type	Sleet, Hail (Freezing Rain or Drizzle)	5
63	Weather Type	Fog/Smog/Smoke	6
63	Weather Type	Blowing Sand/Soil/Dirt/Snow	7
63	Weather Type	Severe Crosswinds	8
63	Weather Type	Other	90
63	Weather Type	Unknown	99

63	Weather Type	Clear	1
63	Weather Type	Cloudy	2
63	Weather Type	Rain	3
63	Weather Type	Snow	4
63	Weather Type	Sleet, Hail (Freezing Rain or Drizzle)	5
63	Weather Type	Fog/Smog/Smoke	6
63	Weather Type	Blowing Sand/Soil/Dirt/Snow	7
63	Weather Type	Severe Crosswinds	8
63	Weather Type	Other	90
63	Weather Type	Unknown	99

63	Weather Type		0
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63	Weather Type		0
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Please see the "Introduction" tab for more information on this tab

Actual Attributes>>			
APPCODEID	APPCODENAME	NAME	VALUE
21	Harmful Event	Pedestrian	8
21	Harmful Event	Pedalcyclist (Bicyclist)	9
21	Harmful Event	Motor Vehicle In Transport	10
21	Harmful Event	Parked Motor Vehicle	11
21	Harmful Event	Struck by Falling, Shifting Cargo or Anything Set in Motion by Motor Vehicle	12
21	Harmful Event	Train - LRT	13
21	Harmful Event	Train - Passenger	14
21	Harmful Event	Train - Cargo	15
21	Harmful Event	Deer	16
21	Harmful Event	Other Animal - Alive at Time of Crash	17
21	Harmful Event	Other Animal - Dead Before Crash	18
21	Harmful Event	Separation of Units	20
21	Harmful Event	Ran Off Roadway Right	21
21	Harmful Event	Ran Off Roadway Left	22
21	Harmful Event	Cross Median	23
21	Harmful Event	Cross Centerline	24
21	Harmful Event	Other - Non Fixed Object	25
21	Harmful Event	Utility Pole/Light Support	28
21	Harmful Event	Traffic Signal or Signal Structure	30
21	Harmful Event	RR / LRT Crossing Device	31
21	Harmful Event	Roadway Sign or Sign Structure	32
21	Harmful Event	Downhill Runaway	33
21	Harmful Event	Fell/Jumped From Motor Vehicle	34
21	Harmful Event	Other Post, Pole or Support	35
21	Harmful Event	Construction or Maintenance Equipment	36
21	Harmful Event	Reentering Roadway	37
21	Harmful Event	Thrown or Falling Object	38
21	Harmful Event	Cargo/Equipment Loss or Shift	39
21	Harmful Event	Bridge Pier or Support	41
21	Harmful Event	Bridge Overhead Structure	42
21	Harmful Event	Bridge Rail	43
21	Harmful Event	Culvert	46
21	Harmful Event	Curb	47
21	Harmful Event	Ditch	48
21	Harmful Event	Embankment	49
21	Harmful Event	Snowbank	50
21	Harmful Event	Other - Non-Motorist	51
21	Harmful Event	Cable Median Barrier	55
21	Harmful Event	Concrete Traffic Barrier	56
21	Harmful Event	Other Traffic Barrier	57
21	Harmful Event	Impact attenuator/ Crash Cushion	60
21	Harmful Event	Guardrail (Face)	61
21	Harmful Event	Guardrail (End)	62
21	Harmful Event	Mailboxes/Posts	67
21	Harmful Event	Hydrant	68
21	Harmful Event	Standing Tree/Shrubbery	69
21	Harmful Event	Fence (Non-Median Barrier)	70
21	Harmful Event	Parking Meter	71
21	Harmful Event	Other - Fixed Object	75
21	Harmful Event	Overturn/Rollover	83

FILTER AND CHART DISPLAYS>>>				
APPCODEID	Attributes	Values>>>>>		
Collision Person, MV, or Non-fixed	Pedestrian	8		
Collision Person, MV, or Non-fixed	Pedalcyclist (Bicyclist)	9		
Collision Person, MV, or Non-fixed	Motor Vehicle In Transport	10	6	7
Collision Person, MV, or Non-fixed	Parked Motor Vehicle	11	81	82
Collision Person, MV, or Non-fixed	Struck by Falling, Shifting Cargo or Anything Set in Motion by Motor Vehicle	12		
Collision Person, MV, or Non-fixed	Train	13	14	15
Collision Person, MV, or Non-fixed	Train - Passenger			
Collision Person, MV, or Non-fixed	Train - Cargo			
Collision Person, MV, or Non-fixed	Deer/Animal	16	17	18
Collision Person, MV, or Non-fixed	Other Animal - Alive at Time of Crash			
Collision Person, MV, or Non-fixed	Other Animal - Dead Before Crash			
Collision Person, MV, or Non-fixed	Other - Non Fixed Object	25	51	5
Collision Person, MV, or Non-fixed	Other - Non Motorist			
Collision Fixed Object	Utility Pole/Light Support	28	26	27
Collision Fixed Object	Traffic Signal or Signal Structure	30		
Collision Fixed Object	RR / LRT Crossing Device	31		
Collision Fixed Object	Roadway Sign or Sign Structure	32		
Collision Fixed Object	Other Post, Pole or Support	35		
Collision Fixed Object	Construction or Maintenance Equipment	36		
Collision Fixed Object	Bridge Pier or Support	41		
Collision Fixed Object	Bridge Overhead Structure	42		
Collision Fixed Object	Bridge Rail	43		
Collision Fixed Object	Culvert	46	40	
Collision Fixed Object	Curb	47		
Collision Fixed Object	Ditch	48		
Collision Fixed Object	Embankment	49	45	
Collision Fixed Object	Snowbank	50		
Collision Fixed Object	Cable Median Barrier	55	53	
Collision Fixed Object	Concrete Traffic Barrier	56		
Collision Fixed Object	Other Traffic Barrier	57		
Collision Fixed Object	Impact attenuator/ Crash Cushion	60		
Collision Fixed Object	Guardrail (Face)	61	54	
Collision Fixed Object	Guardrail (End)	62		
Collision Fixed Object	Mailboxes/Posts	67		
Collision Fixed Object	Hydrant	68		
Collision Fixed Object	Standing Tree/Shrubbery	69		
Collision Fixed Object	Fence (Non-Median Barrier)	70		
Collision Fixed Object	Parking Meter	71		
Collision Fixed Object	Other - Fixed Object	75	65	66
Non-collision Harmful Events	Fell/Jumped From Motor Vehicle	34		
Non-collision Harmful Events	Thrown or Falling Object	38		
Non-collision Harmful Events	Cargo/Equipment Loss or Shift	39		
Non-collision Harmful Events	Overturn/Rollover	83		
Non-collision Harmful Events	Immersion (Full or Partial)	84		
Non-collision Harmful Events	Fire/Explosion	85		
Non-collision Harmful Events	Jackknife	86		
Non-collision Harmful Events	Other Non-Collision	89	80	
Non-Harmful Events	Equipment Failure (blown tire, brake failure, etc.)	19		
Non-Harmful Events	Separation of Units	20		
Non-Harmful Events	Ran Off Roadway Right	21		

21	Harmful Event	Immersion (Full or Partial)	84
21	Harmful Event	Fire/Explosion	85
21	Harmful Event	Jackknife	86
21	Harmful Event	Other Non-Collision	89
21	Harmful Event	Unknown	99
21	Harmful Event		0
21	Harmful Event	COL-RDEQP-SNOWP	1
21	Harmful Event	COL-RDEQP-OTHER	2
21	Harmful Event	COL WITH TRAIN	3
21	Harmful Event	COL ANML NOT DER	4
21	Harmful Event	RUNAWAY VEHICLE	5
21	Harmful Event	COL UNDRIDE-REAR	6
21	Harmful Event	COL UNDRIDE-SIDE	7
21	Harmful Event	LIGHT POLE	26
21	Harmful Event	UTILITY POLE	27
21	Harmful Event	CULVERT/HEADWALL	40
21	Harmful Event	EMBANKMT/DTCH/CB	45
21	Harmful Event	MEDIAN SAFTY BAR	53
21	Harmful Event	GUARDRAIL	54
21	Harmful Event	BUILDING/WALL	65
21	Harmful Event	ROCK OUTCROPS	66
21	Harmful Event	UNKNOWN TYPE FIXED OBJECT	76
21	Harmful Event	UNKN TYP NONCOLL	80
21	Harmful Event	LOSS/SPIL-NONHAZ	81
21	Harmful Event	LOSS/SPIL-HAZMAT	82
21	Harmful Event	UNKNOWN COLLISION TYPE	91
21	Harmful Event	OTHER CRASH TYPE	92
21	Harmful Event	OTHER COLISN TYP	93

Non-Harmful Events	Ran Off Roadway Left	22				
Non-Harmful Events	Cross Median	23				
Non-Harmful Events	Cross Centerline	24				
Non-Harmful Events	Downhill Runaway	33				
Non-Harmful Events	Reentering Roadway	37				
	Unknown	99	91	92	93	0
		0				
	COL-RDEQP-SNOWP					
	COL-RDEQP-OTHER					
	COL WITH TRAIN					
	COL ANML NOT DER					
	RUNAWAY VEHICLE					
	COL UNDRIDE-REAR					
	COL UNDRIDE-SIDE					
	LIGHT POLE					
	UTILITY POLE					
	CULVERT/HEADWALL					
	EMBANKMT/DTCH/CB					
	MEDIAN SAFTY BAR					
	GUARDRAIL					
	BUILDING/WALL					
	ROCK OUTCROPS					
	UNKNOWN TYPE FIXED OBJECT					
	UNKN TYP NONCOLL					
	LOSS/SPIL-NONHAZ					
	LOSS/SPIL-HAZMAT					
	UNKNOWN COLLISION TYPE					
	OTHER CRASH TYPE					
	OTHER COLISN TYP					

## **APPENDIX F**

### **Enclave – Maplewood Project Information**

# Enclave - Maplewood

County Road D & Southlawn Drive, Maplewood, MN 55109  
06/01/22

GROSS AREA - TOTAL	
Level	Area
Level 4	68,735 ft <sup>2</sup>
Level 3	69,281 ft <sup>2</sup>
Level 2	69,281 ft <sup>2</sup>
Level 1	69,606 ft <sup>2</sup>
Level -1	70,411 ft <sup>2</sup>
Grand total	347,314 ft <sup>2</sup>

PARKING		
Level	Type	Count
Level 1	Level 1 Garage	56
Level 1	Surface Parking	94
Level -1	Underground Parking	204
		354

UNIT MIX - GROSS AREA				
Name	Count	Unit Gross Area	Total Area	%
		Main Floor		
<b>0 BR (Studio)</b>				
Unit 0-0	22	581 ft <sup>2</sup>	12,783 ft <sup>2</sup>	9%
	22		12,783 ft <sup>2</sup>	9%
<b>1BR</b>				
Unit 1-0	52	718 ft <sup>2</sup>	37,334 ft <sup>2</sup>	22%
Unit 1-4	69	728 ft <sup>2</sup>	50,221 ft <sup>2</sup>	29%
	121		87,555 ft <sup>2</sup>	50%
<b>2BR</b>				
Unit 2-0	64	1,068 ft <sup>2</sup>	68,336 ft <sup>2</sup>	27%
Unit 2-1	13	1,149 ft <sup>2</sup>	14,937 ft <sup>2</sup>	5%
Unit 2-2	3	1,130 ft <sup>2</sup>	3,390 ft <sup>2</sup>	1%
Unit 2-3	3	1,265 ft <sup>2</sup>	3,794 ft <sup>2</sup>	1%
Unit 2-4	4	1,139 ft <sup>2</sup>	4,557 ft <sup>2</sup>	2%
	87		95,014 ft <sup>2</sup>	36%
<b>3BR</b>				
Unit 3-0	11	1,339 ft <sup>2</sup>	14,729 ft <sup>2</sup>	5%
	11		14,729 ft <sup>2</sup>	5%
Grand total	241		210,081 ft <sup>2</sup>	100%







